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# A high-order FEM formulation for free and forced vibration analysis of a nonlocal nonlinear graded Timoshenko nanobeam based on the weak form quadrature element method

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**Abstract** The purpose of this paper is to provide a high-order finite element method (FEM) formulation of nonlocal nonlinear nonlocal graded Timoshenko based on the weak form quadrature element method (WQEM). This formulation offers the advantages and flexibility of the FEM without its limiting low-order accuracy. The nanobeam theory accounts for the von Kármán geometric nonlinearity in addition to Eringen's nonlocal constitutive models. For the sake of generality, a nonlinear foundation is included in the formulation. The proposed formulation generates high-order derivative terms that cannot be accounted for using regular first- or second-order interpolation functions. Hamilton's principle is used to derive the variational statement which is discretized using WQEM. The results of a WQEM free vibration study are assessed using data obtained from a similar problem solved by the differential quadrature method (DQM). The study shows that WQEM can offer the same accuracy as DQM with a reduced computational cost. Currently the literature describes a small number of high-order numerical forced vibration problems, the majority of which are limited to DQM. To obtain forced vibration solutions using WQEM, the authors propose two different methods to obtain frequency response curves. The obtained results indicate that the frequency response curves generated by either method closely match their DQM counterparts obtained from the literature, and this is despite the low mesh density used for the WQEM systems.

**Keywords** Functionally graded nanobeam · Nonlocal theory · Weak form quadrature element method (WQEM) · Free and forced vibration · Nonlinear von Kármán strain · Frequency response curve

## 1 Introduction

Nanobeams, nanoplates, nanoshells and other small-scale structural elements constitute the building blocks of micro- and nanoelectromechanical systems (MEMS and NEMS), actuators, sensors and atomic force micro-

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scopes [1–3]. The choice of integrating small-scale components is related to exotic mechanical properties and size effects experimentally observed [4–8] at the nanoscale. While these size effects can be accurately captured and studied using molecular dynamics (MD) simulations, the computational cost of MD is generally prohibitive. Hence, higher-order continuum mechanics approaches have been widely adopted as an alternative in the modeling of small-scale structures. Several higher-order continuum theories have been developed, each of which was based on a different perspective of small-scale behavior. However, in general, most of these theories can be classified into three different categories, namely micro-continuum, strain gradient family and nonlocal elasticity theories.

The nonlocal elasticity theory postulates that the stress in a continuum at a given location depends not only on the strain at that location but also on the strains in a finite neighborhood of such point. This dependency on the nonlocal strain is captured by a size effect parameter called the nonlocal parameter. The nonlocal elasticity theory was first proposed by Kroner [9] and then later improved by Eringen and co-workers [10–12]. To simplify the implementation of the theory in practical problems, a differential form was developed [13] based on a specific kernel function. Lately, researchers have explored the possibility of combining nonlocal strain effects with strain gradient theory in a single higher-order theory [14–16] referred to as nonlocal strain gradient theory. These size-dependent theories were exploited to model nanorods [2], nanobeams [1, 17, 18] and nanoplates [19–22]. These simple structures are conventionally modeled based on Euler–Bernoulli beam theory (EBT) and classical plate theory (CPT), respectively. Other models like Timoshenko beam theory (TBT) and first-order shear deformation theory (FSDT) account for shear to model thick beams and plates, respectively, accurately [23, 24].

To overcome the limitations of analytical solutions [24], methods like the finite element method (FEM), the differential quadrature method (DQM), the mesh-free method, the Ritz method, the Galerkin method, etc. were employed to solve small-scale problems and have become the most suitable methods for such problems. In general, numerical techniques are used to solve either the equation of motion or the variational statement. Although developing a solution to the former is generally simpler using collocation methods, solving the variational statement offers several advantages. For example, FEM has weaker regularity requirements (i.e., existence of high-order derivatives) and can easily handle complicated geometries and boundary conditions [25]. These advantages have made FEM the most commonly used method in the analysis of small-scale structures [24].

A few selected studies that use FEM in size-dependent beam problems are summarized in this paragraph, and a more comprehensive review can be found in [24]. Demir and Civalek used a linear nonlocal EBT element in two separate studies [1, 18]. The EBT element is based on Hermite cubic interpolation with two nodes and two degrees of freedom per node. The effect of an elastic matrix was accounted for in both studies. Eltaher and his colleagues developed [26–30] nonlocal EBT elements for both functionally graded (FG) nanobeams [26–28] and homogeneous nanobeams [29, 30]. The EBT element is a two-node element with three degrees of freedom per node: axial and transverse displacements in addition to rotation. The axial displacement is based on a Lagrange linear interpolation, while the transverse displacement is based on Hermite cubic interpolation. Eltaher et al. investigated free vibration problems of FG nanobeams on two separate occasions [26, 28]. In the second paper, the authors reexamined the location of the nanobeam's neutral axis based on the physical neutral surface of FG beams [28]. In addition, Eltaher et al. [27] studied the buckling and bending response of graded nanobeams. Like their FG counterparts, homogeneous nanobeams have received considerable attention in the literature. The free vibration problem of homogeneous nanobeams was also examined by Eltaher et al. [29], while static bending of homogeneous nanobeams was considered by Alshorbagy et al. [30]. Nguyen et al. [31] proposed a mixed formulation consisting of developing a nonlocal mixed beam element to examine the static bending response of homogeneous nanobeams. This two-node element uses Lagrange interpolation for both deflection and bending. The literature also shows fewer nonlocal TBT elements. Reddy and El-Borgi [32] developed a finite element formulation for both nonlocal homogeneous EBT and TBT beams. Their models accounted for moderate rotations using the von Kármán strain nonlinearity. Hence, a nonlinear factor was added to the model. Similarly, the nonlinear EBT element relies on a mix of Lagrange and Hermite cubic interpolation for its axial and transverse displacements, while the nonlinear TBT element uses Lagrange interpolation for all its dependent variables. Later this work was extended to graded nanobeams [33]. Eltaher et al. [34] investigated the buckling and bending behavior of nonlocal graded Timoshenko nanobeams.

According to the literature, there have been several nonlocal beam element formulations. Each was tailored or designed to treat a specific problem. Though some elements were developed for TBT, EBT elements dominates the literature [24]. Technically, linear shape functions are sufficient to design an element model for nonlocal TBT. However, when nonlinear behaviors such as von Kármán strain nonlinearity and nonlinear

elastic foundations are considered, even second-order elements may fall short of addressing all the high-order derivative continuity requirements in the variational statement. This problem was noted by Reddy and El-Borgi [32,33] where the authors chose to neglect high-order derivatives in the variational statement to be able to solve using FEM. Moreover, the literature shows that none of the cited FEM studies have been used to investigate force vibration response and generate frequency response curves (FRC). The lack of forced vibration response prediction in numerical studies can be traced to difficulties in obtaining steady-state responses for a system with a high number of degrees of freedom.

To address the shortcoming related to estimating higher-order derivatives, the problem was generally solved using high-order collocation methods like DQM [35,36] or the quadrature element method (QEM). This method is a high-order method used to solve FEM problems using a single or few high-order elements without the need to explicitly identify shape functions [37]. It relies on DQM matrices and a clever choice of the grid to simplify its implementation and hence may eliminate the need of an assembly subroutine [37]. QEM can be classified into two major families, namely the strong form quadrature element method (SQEM) [38,39] and the weak form quadrature element method (WQEM) [37,40–47]. SQEM is also referred in the literature as the differential quadrature element method [37,48] or the strong formulation finite element method [49]. This approach is formulated similar to the regular DQM [37–39,48,49] with the additional freedom to subdivide the domain into few elements connected by their respective boundary conditions. This allows more flexibility and mitigates the weakness of DQM for discontinuous loading and geometries. On the other hand, WQEM can be formulated in a similar manner as FEM based on the minimum energy principle or the weak form of the integral or the variational statement. It has also been concluded that WQEM converges faster than FEM [50] and it is also more flexible than SQEM since it is essentially a higher-order FEM [37,40,43]. On another note, the applications to two-dimensional thin-plate problems by either DQM or SQEM have been mostly limited to simple domain shapes. Handling more complicated geometries, though theoretically possible [51], may come at the cost of accuracy and efficiency [52]. On the other hand, WQEM, and similar to FEM, can be employed to solve problems with any irregular shapes without any loss of accuracy [40,43]. Furthermore, WQEM stiffness matrix is symmetrical, unlike that of SQEM which may have unstable complex eigenvalues.

WQEM is present in several studies in the literature [46,50,52–63]. Wang et al. [53] performed a comprehensive state-of-the-art review of WQEM and its applications in various engineering applications, including crack propagation [57], 3D domains [55,58], graded media [46,58], beam and plate problems [46,50,52,54,59,63]. Such studies helped extend the range of applications of WQEM. Other studies focused on the accuracy and high convergence rate of WQEM to solve challenging problems which are inaccurately solved using DQM and FEM such as the case of vibration of skewed thin plates [52]. Finally, few other studies focused on solving mathematical challenges such as integration accuracy [46,53], complex form system and system requiring derivative degrees of freedom at the boundary such as the case of slender beams and thin plates [45,46,53].

In spite of the fact that WQEM is useful in estimating higher-order derivatives, few investigators have realized, however, its importance in solving size-dependent continuum mechanics problems [64] and most notably in the case of forced vibration. To fill this gap in the literature, the authors propose to develop a new FEM formulation based on WQEM to model the free and forced vibration response of a nonlocal TBT resting on a nonlinear elastic foundation accounting for moderate rotation through von Kármán strain. The foundation models the interaction between the beam and the medium in which the beam is embedded such as a protein microtubule embedded in a matrix [1] or a carbon nanotube (CNT) embedded in a foundation [3]. To model [65] the forced vibration response, the authors propose two new numerical methods to estimate the frequency response curves of the nanobeam which are validated based on results obtained by the main authors using the differential quadrature method [36]. The closest study to this work is the paper published recently by Jin and Wang [16] who investigated the free vibration response of a linear and classical Timoshenko graded beam using WQEM. As an extension of this paper, the authors added nonlocal and nonlinear effects in addition to forced vibration response.

Following this introduction, the size-dependent equations of motion and the corresponding variational statement for a nonlocal nonlinear graded TBT are established. The following section outlines how the variational statement is discretized using WQEM to obtain the free vibration response of the nanobeam. Section 4 summarizes the WQEM-based forced vibration solution using two different strategies to obtain the frequency response curves. Free and forced vibration results obtained by WQEM are presented and compared with DQM results in Sect. 5. Finally, a summary of this study and concluding remarks are given in Sect. 6.

## 2 Equations of motion and variational statement for a nonlocal TBT

### 2.1 Hamilton's principle

According to Eringen's nonlocal theory [10,13], the nonlocal stress is given by

$$\bar{\sigma} = \int_{\Omega} K(|\mathbf{x}' - \mathbf{x}|, \tau_0) \sigma(\mathbf{x}') d\mathbf{x}' \quad (1)$$

where  $\sigma(\mathbf{x}')$  is the classical macroscopic Cauchy stress tensor at point  $\mathbf{x}'$  and  $K(|\mathbf{x}' - \mathbf{x}|, \tau)$  is the kernel function of the nonlocal modulus,  $|\mathbf{x}' - \mathbf{x}|$  being the distance and  $\tau_0$  being a material parameter that depends on internal and external characteristic lengths. Unlike classical mechanics, this relation stipulates that the stress at a given point in an elastic continuum depends on strains all over the body. An equivalent differential model, based on the exponential kernel, was proposed [13] as

$$(1 - \mu_0^2 \nabla^2) \bar{\sigma} = \mathbf{C} : \boldsymbol{\varepsilon}, \quad \mu_0 = \tau_0 \ell = e_0 a \quad (2)$$

where  $e_0$  is a material constant,  $\nabla^2$  is the Laplacian operator, and  $a$  and  $\ell$  are the internal and external characteristic lengths, respectively. It is usually assumed that the nonlocal size effect is only significant along the  $x$ -axis of the nanobeam which is along its longitudinal direction. In light of this assumption, Eq. (2) is reduced to the following:

$$\left(1 - \mu_0^2 \frac{\partial^2}{\partial x^2}\right) \bar{\sigma}_{xx} = E \varepsilon_{xx} \quad (3a)$$

$$\left(1 - \mu_0^2 \frac{\partial^2}{\partial x^2}\right) \bar{\sigma}_{xz} = G \gamma_{xz} \quad (3b)$$

where  $\nabla^2$  is reduced to  $\partial^2/\partial x^2$ ,  $E$  is the elastic modulus of the beam and  $G$  is its shear modulus.

It is worth noting at this point that the transformation from the integral to the differential form of the nonlocal model comes with a paradox for beam bending problems with an exponential nonlocal kernel. In fact, Fernandez-Saez et al. [66] and Romano et al. [67] reported that this transformation yields a relationship that must be satisfied between the bending moment and the spatial derivative of the bending moment at the boundaries. The bending moment obtained from the solution of the differential equation should be checked to ensure the obtained solution is also a solution to the integral form of the model. This is easily done for problems with displacement-type boundary conditions, since the bending moment will be the solution of a second-order differential equation, and the constants of integration can be used to satisfy the bending moment boundary conditions. However, it should be noted that the integral form is incapable to model local effects at boundaries, which may result in some discrepancies between the actual and simulated bending moment at the boundary. Knowing that neither the integral form nor the differential form can solve all possible discrepancies at the boundaries, the differential form is selected in this study. These arguments were also used by the last two authors in a previous paper for choosing the nonlocal differential model [68].

A Timoshenko beam resting on a nonlinear foundation shown in Fig. 1 is considered in this study. Within the context of the small displacement and small deformation theory and only accounting for bending in the  $x$ - $z$  plane, the components of the displacement field in a TBT model are assumed to be written as

$$u_x(x, z, t) = u(x, t) + z \phi_x(x, t) \quad (4a)$$

$$u_z(x, z, t) = w(x, t) \quad (4b)$$

Accounting for von Kármán strain, the Green–Lagrange strain components can be expressed as

$$\varepsilon_{xx} = \varepsilon_{xx}^{(0)} + z \varepsilon_{xx}^{(1)}, \quad \gamma_{xz} = \gamma_{xz}^{(0)} \quad (5)$$

where

$$\varepsilon_{xx}^{(0)} = \frac{\partial u}{\partial x} + \frac{1}{2} \left( \frac{\partial w}{\partial x} \right)^2, \quad \varepsilon_{xx}^{(1)} = \frac{\partial \phi_x}{\partial x}, \quad \gamma_{xz}^{(0)} = \phi_x + \frac{\partial w}{\partial x} \quad (6)$$

Hamilton's principle for the current nonlinear Timoshenko nanobeam can be written as

$$\int_{t_1}^{t_2} (\delta K - \delta U + \delta W) dt = 0 \quad (7)$$

where  $\delta K$  is the variation of the kinetic energy,  $\delta U$  is the variation of strain energy and  $\delta W$  is the variation of the external work. These terms can be expressed as

$$\delta K = \int_0^l (m_0 \dot{u} \delta \dot{u} + m_2 \dot{\phi}_x \delta \dot{\phi}_x + m_0 \dot{w} \delta \dot{w}) dx \quad (8a)$$

$$\begin{aligned} \delta U &= \int_0^l \int_A (\sigma_{xx} \delta \varepsilon_{xx} + \sigma_{xz} \delta \gamma_{xz}) dA dx + \int_0^l F_v \delta w dx \\ &= \int_0^l \left\{ M_{xx}^{(0)} \left( \frac{\partial \delta u}{\partial x} + \frac{\partial w}{\partial x} \frac{\partial \delta w}{\partial x} \right) + M_{xx}^{(1)} \frac{\partial \delta \phi_x}{\partial x} + M_{xz}^{(0)} \left( \delta \phi_x + \frac{\partial \delta w}{\partial x} \right) + F_v \delta w \right\} dx \end{aligned} \quad (8b)$$

$$\delta W = - \int_0^l (q \delta w) dx \quad (8c)$$

where  $q$  is the distributed transverse load and  $F_v = \mu_f \dot{w}$  represents the damping force assumed to be proportional to the velocity  $\dot{w}$  wherein  $\mu_f$  is the damping coefficient. It is worth noting that there is no damping associated with rotation since the beam is considered elastic and not viscoelastic [69]. Finally,  $M_{ij}^{(k)}$  is a stress resultant defined as

$$M_{ij}^{(k)} = \int_A z^k \sigma_{ij} dA \quad (9)$$

## 2.2 Equations of motion

Substituting Eqs. (8a), (8b) and (8c) into Hamilton's principle (7), and then integrating by parts, yields the motion equations of the nanobeam which can be written as

$$- \frac{\partial M_{xx}^{(0)}}{\partial x} + m_0 \frac{\partial^2 u}{\partial t^2} = 0 \quad (10a)$$

$$- \frac{\partial}{\partial x} \left( M_{xz}^{(0)} + M_{xx}^{(0)} \frac{\partial w}{\partial x} \right) + \mu_f \frac{\partial w}{\partial t} + m_0 \frac{\partial^2 w}{\partial t^2} = q \quad (10b)$$

$$M_{xz}^{(0)} - \frac{\partial}{\partial x} M_{xx}^{(1)} + m_2 \frac{\partial^2 \phi_x}{\partial t^2} = 0 \quad (10c)$$

For a beam graded in the  $z$  direction, the elastic and shear moduli, appearing in Eqs. (3a) and (3b), are assumed to follow the power-law function below [27,46]

$$E(z) = (E_U - E_L) \left( \frac{z}{h} + \frac{1}{2} \right)^{n_k} + E_L, \quad G(z) = K_s E(z) \quad (11)$$

where the subscripts  $U$  and  $L$  designate the upper and lower faces of the beam. Here,  $h$  is the thickness of the beam and  $n_k$  is the material gradation index. In light of the above equations, the nanobeam is considered to be nonhomogeneous with an isotropic stress-strain law. Combining (3a) and (3b) with Eq. (9) yields:

$$M_{xx}^{(0)} - \mu_0^2 \frac{\partial^2 M_{xx}^{(0)}}{\partial x^2} = \tilde{A} \varepsilon_{xx} = \tilde{A} \left[ \frac{\partial u}{\partial x} + \frac{1}{2} \left( \frac{\partial w}{\partial x} \right)^2 \right] \quad (12a)$$

$$M_{xx}^{(1)} - \mu_0^2 \frac{\partial^2 M_{xx}^{(1)}}{\partial x^2} = \tilde{B} \varepsilon_{xx} = \tilde{B} \frac{\partial \phi_x}{\partial x} \quad (12b)$$

$$M_{xz}^{(0)} - \mu_0^2 \frac{\partial^2 M_{xz}^{(0)}}{\partial x^2} = \tilde{G} \gamma_{xz}^{(0)} = \tilde{G} \left[ \phi_x + \frac{\partial w}{\partial x} \right] \quad (12c)$$

in which

$$\tilde{A} = \int_A E(z) dA, \quad \tilde{B} = \int_A z^2 E(z) dA, \quad \tilde{G} = \int_A G(z) dA \quad (13)$$

Next, using the technique developed by Nayfeh and Pai [70], the axial displacement  $u$  is eliminated from the equations of motion. To apply this technique, the following assumptions are adopted: (i) The beam is supported at its both boundary points such that  $u(0) = u(l) = 0$  and (ii) the longitudinal acceleration  $m_0 \partial^2 u / \partial t^2$  and the corresponding velocity are assumed to be very small and hence can be neglected. Applying these assumptions yields the following expression of  $M_{xx}^{(0)}$  which can be written as:

$$M_{xx}^{(0)} = \tilde{A} \varepsilon_{xx}^{(0)} = C_2(t) = \frac{1}{2S} \left\{ \int_0^l \left( \frac{\partial w}{\partial x} \right)^2 dx \right\} \quad (14)$$

where  $S = \int_0^l \frac{1}{A} dx = \frac{l}{A}$ . Further details about this simplification can be found in [36]. With further manipulations, it can be shown that the nonlocal stress resultants can be written entirely in terms of displacements

$$M_{xx}^{(0)} = \tilde{A} \varepsilon_{xx}^{(0)} = C_2 \quad (15a)$$

$$M_{xz}^{(0)} = \mu_0^2 \left\{ -\frac{\partial^2}{\partial x^2} \left[ \frac{\partial w}{\partial x} C_2 \right] - \frac{\partial q}{\partial x} + \mu_f \frac{\partial^2 w}{\partial x \partial t} + m_0 \frac{\partial^3 w}{\partial x \partial t^2} \right\} + \tilde{G} \left( \phi_x + \frac{\partial w}{\partial x} \right) \quad (15b)$$

$$M_{xx}^{(1)} = \mu_0^2 \left\{ -\frac{\partial}{\partial x} \left[ \frac{\partial w}{\partial x} C_2 \right] - q + \mu_f \frac{\partial w}{\partial t} + m_0 \frac{\partial^2 w}{\partial t^2} + m_2 \frac{\partial^3 \phi_x}{\partial x \partial t^2} \right\} + \tilde{B} \frac{\partial \phi_x}{\partial x} \quad (15c)$$

Finally, substituting the above equations into (10b) and (10c) yields the following reduced equations of motion:

$$\left[ 1 - \mu_0^2 \frac{\partial^2}{\partial x^2} \right] \left\{ m_0 \frac{\partial^2 w}{\partial t^2} + \mu_f \frac{\partial w}{\partial t} - \frac{\partial^2 w}{\partial x^2} \left\{ \frac{1}{2S} \int_0^l \left( \frac{\partial w}{\partial x} \right)^2 dx \right\} - q \right\} - \tilde{G} \frac{\partial}{\partial x} \left( \phi_x + \frac{\partial w}{\partial x} \right) = 0 \quad (16a)$$

$$\left[ 1 - \mu_0^2 \frac{\partial^2}{\partial x^2} \right] \left\{ m_2 \frac{\partial^2 \phi_x}{\partial t^2} \right\} + \tilde{G} \left( \phi_x + \frac{\partial w}{\partial x} \right) - \tilde{B} \frac{\partial^2 \phi_x}{\partial x^2} = 0 \quad (16b)$$

The nonlinear elastic foundation is assumed to be a transversely acting stiffness. Hence, in the case of a forced vibration load,  $q(x, t)$  is given by

$$q(x, t) = -k_L w - k_{NL} w^3 + k_s \frac{\partial^2 w}{\partial x^2} + F(x) \cos(\omega t) \quad (17)$$

where  $k_L$ ,  $k_{NL}$  and  $k_s$  are, respectively, the linear, nonlinear and shear coefficients of the nonlinear medium in which the beam is embedded and therefore represent the effect of the surrounding medium. It is also worth noting that this model is a generalization of the linear models known as Winkler [71] and Pasternak [72] foundations, although a more complex model could be a viscoelastic foundation [73]. Previous papers published by the main authors [35, 36] confirm that the effect of the surrounding material is crucial and the nonlinear stiffness parameter  $k_{NL}$  appearing in Eq. (17) plays a dominant role in the response of the nanobeam. Therefore, it was decided to adopt the current nonlinear foundation in this study rather than the linear classical Winkler-type and Pasternak-type foundations. Finally,  $F(x)$  and  $\omega$ , appearing in the above equation, designate, respectively, the forcing function amplitude and frequency. Finally, the amplitude is set to zero for free vibration case.

For scaling purposes, the following normalization is utilized:

$$\xi = \frac{x}{l}, \quad \hat{w} = \frac{w}{r}, \quad \hat{\phi} = \frac{l}{r} \phi_x, \quad s^2 = \frac{\tilde{B}}{l^2 \tilde{G}}, \quad \kappa_0 = \frac{r^2 \tilde{A}}{2 \tilde{B}} \quad (18a)$$

$$\tau = t \frac{1}{l^2} \sqrt{\frac{\tilde{B}}{m_0}}, \quad r = \sqrt{\frac{I}{A}}, \quad \hat{\mu}_0 = \frac{\mu_0}{l}, \quad m = \frac{m_2}{l^2 m_0}, \quad \hat{\mu}_f = \frac{\mu_f l^2}{\sqrt{m_0 \tilde{B}}} \quad (18b)$$

$$\hat{k}_L = k_L \frac{l^4}{\tilde{B}}, \quad \hat{k}_{NL} = k_{NL} \frac{r^2 l^4}{\tilde{B}}, \quad \hat{k}_s = k_s \frac{l^2}{\tilde{B}}, \quad \hat{F}(\xi) = F(x) \frac{l^4}{\tilde{B}}, \quad \hat{\omega} = \omega l^2 \sqrt{\frac{m_0}{\tilde{B}}} \quad (18c)$$

This yields the following nondimensional equations of motion:

$$\left[ 1 - \hat{\mu}_0^2 \frac{\partial^2}{\partial \xi^2} \right] \left( \frac{\partial^2 \hat{w}}{\partial \tau^2} + \hat{\mu}_f \frac{\partial \hat{w}}{\partial \tau} - \kappa_0 \frac{\partial^2 \hat{w}}{\partial \xi^2} \int_0^1 \left( \frac{\partial \hat{w}}{\partial \xi} \right)^2 d\xi - \hat{q} \right) - \frac{1}{s^2} \frac{\partial}{\partial \xi} \left( \hat{\phi} + \frac{\partial \hat{w}}{\partial \xi} \right) = 0 \quad (19a)$$

$$\left[ 1 - \hat{\mu}_0^2 \frac{\partial^2}{\partial \xi^2} \right] \left( m \frac{\partial^2 \hat{\phi}}{\partial \tau^2} \right) + \frac{1}{s^2} \left( \hat{\phi} + \frac{\partial \hat{w}}{\partial \xi} \right) - \frac{\partial^2 \hat{\phi}}{\partial \xi^2} = 0 \quad (19b)$$

where  $\hat{q} = \hat{k}_s \frac{\partial^2 \hat{w}}{\partial \xi^2} - \hat{k}_L \hat{w} - \hat{k}_{NL} \hat{w}^3 + \hat{F}(\xi) \cos(\hat{\omega} \tau)$  and

$$\begin{aligned} \left( \hat{q} - \hat{\mu}_0^2 \frac{\partial^2 \hat{q}}{\partial \xi^2} \right) &= \hat{k}_s \left( \frac{\partial^2 \hat{w}}{\partial \xi^2} - \hat{\mu}_0^2 \frac{\partial^4 \hat{w}}{\partial \xi^4} \right) - \hat{k}_L \left( \hat{w} - \hat{\mu}_0^2 \frac{\partial^2 \hat{w}}{\partial \xi^2} \right) - \hat{k}_{NL} \left[ \hat{w}^3 - 3\hat{\mu}_0^2 \left( 2\hat{w} \left( \frac{\partial \hat{w}}{\partial \xi} \right)^2 + \hat{w}^2 \frac{\partial^2 \hat{w}}{\partial \xi^2} \right) \right] \\ &+ \left( \hat{F}(\xi) - \hat{\mu}_0^2 \frac{\partial^2 \hat{F}}{\partial \xi^2} \right) \cos(\hat{\omega} \tau) \end{aligned}$$

For the hinged–hinged (HH) case, the following boundary conditions must be satisfied at the ends of the beam, i.e., at both  $\xi = 0$  and  $\xi = 1$ :

$$\hat{w} = 0, \quad \hat{M}_{xx}^{(1)} = 0 \quad (20)$$

which is equivalent to

$$\hat{w} = 0 \quad (21a)$$

$$\frac{\partial \hat{\phi}}{\partial \xi} - \kappa_0 \hat{\mu}_0^2 \left( \int_0^1 \left( \frac{\partial \hat{w}}{\partial \xi} \right)^2 d\xi \right) \frac{\partial^2 \hat{w}}{\partial \xi^2} + \hat{\mu}_0^2 \left[ \frac{\partial^2 \hat{w}}{\partial \tau^2} + m \frac{\partial^3 \hat{\phi}}{\partial \xi \partial \tau^2} + \hat{\mu}_f \frac{\partial \hat{w}}{\partial \tau} - \hat{q} \right] = 0 \quad (21b)$$

A clamped–clamped (CC) nanobeam must satisfy the following boundary conditions at  $\xi = 0$  and at  $\xi = 1$ :

$$\hat{w} = 0, \quad \hat{\phi} = 0 \quad (22)$$

### 2.3 Variational statement

The aim of this study is to formulate a high-order variational method. To this end, Eq. (15) is substituted into Eqs. (8a), (8b) and (8c). The resulting expressions are then substituted into the expression of Hamilton's principle (7). Finally, integrating (8a) by parts, the variational formulation can be written as a function of displacements as follows:

$$\begin{aligned} &\int_0^l [m_0 \ddot{w} + \mu_f \dot{w} - q] \delta w \\ &+ \left[ \mu_0^2 m_0 \ddot{w}' + \mu_f \mu_0^2 \dot{w}' - \mu_0^2 q' + \tilde{G} (w' + \phi_x) + (w' - \mu_0^2 w''') \frac{1}{2S} \int_0^l w'^2 dx \right] \delta w' \\ &+ \left[ \mu_0^2 m_0 \ddot{w}' + m_2 \ddot{\phi}_x + \mu_f \mu_0^2 \dot{w}' + \tilde{G} (w' + \phi_x) - \mu_0^2 \left( q' + w''' \left( \frac{1}{2S} \int_0^l w'^2 dx \right) \right) \right] \delta \phi_x \\ &+ \left[ \mu_0^2 m_0 \ddot{w} + \mu_0^2 m_2 \ddot{\phi}_x + \mu_f \mu_0^2 \dot{w} + \tilde{B} \phi_x' - \mu_0^2 \left( q + w'' \left( \frac{1}{2S} \int_0^l w'^2 dx \right) \right) \right] \delta \phi_x' dx \end{aligned} \quad (23)$$

which can be rewritten in scalar product form as follows:

$$\int_0^l \begin{pmatrix} \delta w \\ \delta w' \\ \delta \phi_x \\ \delta \phi_x' \end{pmatrix}^T \cdot \left( \frac{\partial^2}{\partial t^2} \begin{pmatrix} m_0 w \\ \mu_0^2 m_0 w' \\ \mu_0^2 m_0 w' + m_2 \phi_x \\ \mu_0^2 m_0 w + \mu_0^2 m_2 \phi_x' \end{pmatrix} + \mu_f \frac{\partial}{\partial t} \begin{pmatrix} w \\ \mu_0^2 w' \\ \mu_0^2 w' \end{pmatrix} + \begin{pmatrix} -\mu_0^2 q' + \tilde{G}(w' + \phi_x) + (w' - \mu_0^2 w''') \frac{1}{2S} \int_0^l w'^2 dx \\ \tilde{G}(w' + \phi_x) - \mu_0^2 (q' + w''') \left( \frac{1}{2S} \int_0^l w'^2 dx \right) \\ \tilde{B} \phi_x' - \mu_0^2 (q + w'') \left( \frac{1}{2S} \int_0^l w'^2 dx \right) \end{pmatrix} \right) dx \quad (24)$$

Since only one element will be used to model the nanobeam, the external forces at the boundaries of the beam element are basically reaction forces. Hence, the work of external forces at the boundaries of the nanobeam is zero and does not need to be added to the variational statement [32].

Examining the above variational statement reveals that  $w$  is raised to the third derivative in several terms. One alternative used by Reddy et al. [33] is to neglect these terms and adopt a quadratic finite element model that does not account for all mechanical aspects of the system. A better alternative is to raise the order of the finite element model and one viable approach is the p-version of the finite element method. However, a simpler alternative adopted in this paper is to deploy WQEM to discretize the system, which in addition brings high-order accuracy.

Finally, utilizing the normalized variables in (18), the normalized variational statement can then be expressed as follows:

$$\int_0^1 \begin{pmatrix} \delta \hat{w} \\ \delta \hat{w}' \\ \delta \hat{\phi} \\ \delta \hat{\phi}' \end{pmatrix}^T \cdot \left( \frac{\partial^2}{\partial \tau^2} \begin{pmatrix} \hat{w} \\ \hat{\mu}_0^2 \hat{w}' \\ \hat{\mu}_0^2 \hat{w}' + m \hat{\phi} \\ \hat{\mu}_0^2 \hat{w} + \mu_0^2 m \hat{\phi}' \end{pmatrix} + \hat{\mu}_f \frac{\partial}{\partial \tau} \begin{pmatrix} \hat{w} \\ \hat{\mu}_0^2 \hat{w}' \\ \hat{\mu}_0^2 \hat{w}' \end{pmatrix} + \begin{pmatrix} -\hat{\mu}_0^2 \hat{q}' + \frac{1}{s^2} (\hat{w}' + \hat{\phi}) + \kappa_0 (\hat{w}' - \hat{\mu}_0^2 \hat{w}''') \int_0^1 \hat{w}'^2 d\xi \\ \frac{1}{s^2} (\hat{w}' + \hat{\phi}) - \hat{\mu}_0^2 (\hat{q}' + \kappa_0 \hat{w}''') \int_0^1 \hat{w}'^2 d\xi \\ \hat{\phi}' - \hat{\mu}_0^2 (\hat{q} + \kappa_0 \hat{w}'') \int_0^1 \hat{w}'^2 d\xi \end{pmatrix} \right) d\xi \quad (25)$$

The above nondimensional variational statement is the one subsequently solved in the free and forced vibration studies (Sects. 4 and 5). Furthermore, it is obvious that the highest derivative  $\hat{w}'''$  appearing in Eqs. (24) and (25) cannot be accounted for based on a regular FEM formulation even with second-order elements. The formulation of classical FEM requires the definition of shape functions whose order defines the element's order. Modifying the element order to accommodate higher derivatives (or to increase elements precision) requires the development of a brand new formulation. This locks the order FEM element at the formulation stage. WQEM, however, does not require an explicit computation of shape functions or their derivatives [37]. This allows the use of adaptive order of precision and hence can avoid any unnecessary approximations.

### 3 Free vibration WQEM formulation

To simplify the computation of the discretized system and later write the variational statement in matrix form, the following integral is first approximated  $\int_a^b f(\xi)^{(m)} g(\xi)^{(k)} d\xi$ . Here,  $f(\xi)$  and  $g(\xi)$  are arbitrary functions that can be interpolated using a Lagrange polynomial basis of order  $n$  and  $f(\xi)^{(m)}$  and  $g(\xi)^{(k)}$  are, respectively, the  $m$ th-order and  $k$ th-order derivatives of  $f(\xi)$  and  $g(\xi)$  with respect to  $\xi$ . To this end, an  $n$ -node mesh has to be selected and the integral can be evaluated as

$$\begin{aligned} \int_a^b f(\xi)^{(m)} g(\xi)^{(k)} d\xi &\approx \sum_{i=1}^n [\omega_\xi]_i f^{(m)}(\xi_i) g^{(k)}(\xi_i) \\ &= \begin{bmatrix} [\omega_\xi]_1 f^{(m)}(\xi_1) \\ \vdots \\ [\omega_\xi]_n f^{(m)}(\xi_n) \end{bmatrix}^T \cdot \begin{bmatrix} g^{(k)}(\xi_1) \\ \vdots \\ g^{(k)}(\xi_n) \end{bmatrix} \end{aligned}$$



$$= \left( \begin{array}{c} [\omega_\xi] \begin{bmatrix} f^{(m)}(\xi_1) \\ \vdots \\ f^{(m)}(\xi_n) \end{bmatrix} \\ \underbrace{\hspace{10em}}_{[f^{(m)}(\{\xi\})]} \end{array} \right)^T \cdot \begin{array}{c} \begin{bmatrix} g^{(k)}(\xi_1) \\ \vdots \\ g^{(k)}(\xi_n) \end{bmatrix} \\ \underbrace{\hspace{10em}}_{[g^{(k)}(\{\xi\})]} \end{array} \quad (26)$$

where  $\xi_i (1 \leq i \leq n)$  are the mesh coordinates,  $\{\xi\}$  is the mesh coordinate vector and  $[\omega_\xi]_i$  are the integral quadrature weights relative to  $\xi_i, (1 \leq i \leq n)$ . In the above equation, the term  $[\omega_\xi][f(\xi)]$  is an element by element multiplication and  $[f^{(m)}(\{\xi\})]^T \cdot [g^{(k)}(\{\xi\})]$  is a regular matrix (or vector) multiplication. Using DQM matrices, Eq. (26) can be further simplified as follows:

$$\begin{aligned} \int_a^b f(\xi)^{(m)} g(\xi)^{(k)} d\xi &\approx \left( \begin{array}{c} [\omega_\xi][M_m] \cdot \begin{bmatrix} f(\xi_1) \\ \vdots \\ f(\xi_n) \end{bmatrix} \\ \underbrace{\hspace{10em}}_{[f^{(m)}(\{\xi\})]} \end{array} \right)^T \cdot \begin{array}{c} [M_k] \cdot \begin{bmatrix} g(\xi_1) \\ \vdots \\ g(\xi_n) \end{bmatrix} \\ \underbrace{\hspace{10em}}_{[g^{(k)}(\{\xi\})]} \end{array} \\ &= \begin{array}{c} \begin{bmatrix} f(\xi_1) \\ \vdots \\ f(\xi_n) \end{bmatrix} \\ \underbrace{\hspace{10em}}_{[f(\{\xi\})]} \end{array} \left( [\omega_\xi][M_m] \right)^T \cdot [M_k] \cdot \begin{array}{c} \begin{bmatrix} g(\xi_1) \\ \vdots \\ g(\xi_n) \end{bmatrix} \\ \underbrace{\hspace{10em}}_{[g(\{\xi\})]} \end{array} \end{array} \quad (27)$$

where  $[M_m]$  is the  $m$ th-order differentiation DQM matrix whose expression is given in ‘‘Appendix’’ 7.  $M_0$  designates the identity matrix which has the same order as the DQM matrices. Here, the notation  $[\omega_\xi][M_m]$  indicates that the elements of  $[\omega_\xi]$  multiply the rows of  $[M_m]$ . It will be referred to later simply as  $[\omega_\xi M_m]$ . Using (27), the integrals in (25) can be discretized and written in a matrix form. Technically,  $Y_1$  and  $Y_2$  are defined as vectors of nodal displacements such as  $[Y_1]_i = \hat{w}_i(\tau)$  and  $[Y_2]_i = \hat{\phi}_i(\tau)$  where  $(1 \leq i \leq n)$ . In addition,  $\delta Y_1$  and  $\delta Y_2$  are vectors of virtual displacements such as  $[\delta Y_1]_i = \delta \hat{w}_i(\tau)$  and  $[\delta Y_2]_i = \delta \hat{\phi}_i(\tau)$ . Finally, the general displacement vector  $Y$  is defined as  $Y = \begin{bmatrix} Y_1 \\ Y_2 \end{bmatrix}$ , while the velocity and acceleration vectors are denoted by  $\dot{Y}$  and  $\ddot{Y}$ , respectively.

A WQEM discretization is utilized to obtain the free vibration solution of the nanobeam. The mesh coordinates  $\xi_i (i = 1, \dots, n)$  for  $n$  nodes are chosen based on the Gauss–Lobatto–Legendre (GLL) quadrature grid which yields an integration accuracy up to a polynomial of degree  $(2n - 3)$  [37,45]. For a general linear TBT, this should result in a fully integrated stiffness matrix and reduced integrated mass matrix [37,45]. Applying the spatial discretization in (27) to the variational statement (25) yields the following:

$$\begin{aligned} &[\omega_\xi M_0]^T \cdot (M_0 \cdot \ddot{Y}_1 - \hat{q}) + \hat{\mu}_0^2 [\omega_\xi M_1]^T \cdot (M_1 \cdot \ddot{Y}_1 - \hat{q}') + \\ &[\omega_\xi M_1]^T \cdot \left( iC (M_1 \cdot Y_1 - \hat{\mu}_0^2 M_3 \cdot Y_1) + \frac{1}{s^2} (M_0 \cdot Y_2 + M_1 \cdot Y_1) \right) = 0 \end{aligned} \quad (28a)$$

$$\begin{aligned} &[\omega_\xi M_0]^T \cdot \left( \frac{1}{s^2} (M_0 \cdot Y_2 + M_1 \cdot Y_1) + m M_0 \cdot \ddot{Y}_2 + \hat{\mu}_0^2 M_1 \cdot \ddot{Y}_1 - \hat{\mu}_0^2 iC M_3 \cdot Y_1 - \hat{q}' \right) + \\ &[\omega_\xi M_1]^T \cdot (M_1 \cdot Y_2 + \hat{\mu}_0^2 (m M_1 \cdot \ddot{Y}_2 + M_0 \cdot \ddot{Y}_1 - iC M_2 \cdot Y_1 - \hat{q})) = 0 \end{aligned} \quad (28b)$$

where

$$\begin{aligned} iC &= \kappa_0 (\{\omega_\xi\} \cdot \{(M_1 \cdot Y_1)^2\}) \\ \hat{q} &= \hat{k}_s M_2 \cdot Y_1 - \hat{k}_L M_0 \cdot Y_1 - \hat{k}_{NL} (M_0 \cdot Y_1)^3 \\ \hat{q}' &= \hat{k}_s M_3 \cdot Y_1 - \hat{k}_L M_1 \cdot Y_1 - 3\hat{k}_{NL} (M_0 \cdot Y_1)^2 M_1 \cdot Y_1 \end{aligned}$$

and  $\omega_\xi$  is an integral quadrature weight coefficient vector compatible with the GLL grid.  $M_2 M_1$  or  $M_1^2$  is an element by element product (i.e., the Hadamard product) of either two vectors or two matrices. The matrix form in Eqs. (28a) and (28b) produces a system of  $2n$  coupled differential equations,  $n$  for each degree of freedom. Hinged–hinged (HH) and clamped–clamped (CC) beams are the only boundary conditions considered herein. Since all boundary conditions present in this study are homogeneous, only essential boundary conditions need to be explicitly stated for variational methods. These boundary conditions are given by

$$\begin{aligned} [Y_1]_{k_b} &= 0 \\ - & - - \\ [Y_1]_{k_b} &= 0 \\ [Y_2]_{k_b} &= 0 \end{aligned} \quad (29)$$

where  $k_b$  is either 1 or  $n$ . The system of Eqs. (28a) and (28b) is then reduced to its basic degrees of freedom using the procedure outlined in [35,64,65,74]

$$\left[ M_{Sys}^{\{R\}} \right] \cdot [\ddot{Y}^{\{R\}}] + \left[ K_{Sys}^{\{R\}}(Y^{\{R\}}) \right] \cdot [Y^{\{R\}}] = \{0\} \quad (30)$$

Here, the superscript  $\{R\}$  denotes the reduced formulation of the system,  $M_{Sys}$  is the mass matrix and  $K_{Sys}(Y)$  is the nonlinear stiffness matrix. To obtain the eigenvalues,  $Y$  is assumed to have the following form  $Y = \tilde{Y} e^{i\omega t}$ . Then, Eq. (30) is rewritten in the following form:

$$\left( \left[ K_{Sys}^{\{R\}}(Y^{\{R\}}) \right] - \omega^2 \left[ M_{Sys}^{\{R\}} \right] \right) \cdot \tilde{Y}^{\{R\}} = 0 \quad (31)$$

To obtain the linear natural eigen-system,  $Y^{\{R\}}$  is set to  $\{0\}$  in  $\left[ K_{Sys}^{\{R\}}(Y^{\{R\}}) \right]$  in (31). The  $i$ th nonlinear natural frequency is obtained through an iterative process which starts with the  $i$ th linear eigenvector to evaluate  $\left[ K_{Sys}^{\{R\}}(Y^{\{R\}}) \right]$ . The newly estimated eigenvector is used to update  $\left[ K_{Sys}^{\{R\}}(Y^{\{R\}}) \right]$  until reaching convergence. Since the nonlinear frequency is amplitude dependent, the  $i$ th eigenvector must always be scaled relative to the mode shape of  $w(x, t)$  in order to keep the mode shape's amplitude constant.

#### 4 Forced vibration WQEM formulation

The free vibration studies related to the problems similar to the one under investigation largely outnumbered their force vibration counterparts in the literature. In fact, there are limited number studies of forced vibration involving DQM [36,65,74,75], especially in size-dependent mechanics. A number of nonclassical mechanics WQEM studies are extremely rare and are focused on the free vibration response [64]. To the best of the authors knowledge, there have been no forced vibration WQEM studies in the literature similar to the one present with DQM. To fill this gap, two force vibration methods are proposed in this section. Each of the proposed methods aims at finding the periodic steady-state solution of the system for different excitation frequencies. For this aim, the time is discretized using a periodic grid and periodic derivation matrices. However, adding a time discretization is equivalent to adding another dimension to the problem with consequential important computational cost. Hence, knowing that the forcing term should only excite a few modes, it is necessary to reduce the spatial degrees of freedom. In this section, each proposed method introduces a different approach to perform this task.

Adding the forcing and the damping terms to (28a) and (28b) yields the following WQEM formulation of the variational statement for the forced vibration case:

$$\begin{aligned} & \left[ \omega_\xi M_0 \right]^T \cdot (M_0 \cdot \dot{Y}_1 + \hat{\mu}_f M_0 \cdot \dot{Y}_1 - \hat{q}) + \hat{\mu}_0^2 \left[ \omega_\xi M_1 \right]^T \cdot (M_1 \cdot \ddot{Y}_1 + \hat{\mu}_f M_1 \cdot \dot{Y}_1 - \hat{q}') + \\ & \left[ \omega_\xi M_1 \right]^T \cdot \left( iC (M_1 \cdot Y_1 - \hat{\mu}_0^2 M_3 \cdot Y_1) + \frac{1}{s^2} (M_0 \cdot Y_2 + M_1 \cdot Y_1) \right) = 0 \end{aligned} \quad (32a)$$

$$\begin{aligned} & - - - - \\ & \left[ \omega_\xi M_0 \right]^T \cdot \left( \frac{1}{s^2} (M_0 \cdot Y_2 + M_1 \cdot Y_1) + m M_0 \cdot \ddot{Y}_2 + \hat{\mu}_0^2 (M_1 \cdot \ddot{Y}_1 + \hat{\mu}_f M_1 \cdot \dot{Y}_1 - iC M_3 \cdot Y_1) - \hat{q}' \right) + \\ & \left[ \omega_\xi M_1 \right]^T \cdot (M_1 \cdot Y_2 + \hat{\mu}_0^2 (m M_1 \cdot \ddot{Y}_2 + M_0 \cdot \ddot{Y}_1 + \hat{\mu}_f M_0 \cdot \dot{Y}_1 - iC M_2 \cdot Y_1 - \hat{q})) = 0 \end{aligned} \quad (32b)$$

where

$$\begin{aligned}
i\mathcal{C} &= \kappa_0 \left( \{\omega_\xi\} \cdot \{(M_1.Y_1)^2\} \right) \\
\hat{q} &= \hat{k}_s M_2.Y_1 - \hat{k}_L M_0.Y_1 - \hat{k}_{NL} (M_0.Y_1)^3 + \hat{F} \cos(\hat{\omega}\tau) \\
\hat{q}' &= \hat{k}_s M_3.Y_1 - \hat{k}_L M_1.Y_1 - 3\hat{k}_{NL} (M_0.Y_1)^2 M_1.Y_1 + M_1.\hat{F} \cos(\hat{\omega}\tau)
\end{aligned}$$

in which  $\hat{\mu}_f$  is the normalized damping coefficient and  $\hat{F}$  denotes the normalized discretized force distribution.

#### 4.1 WQEM formulation using a mode shape interpolation basis

The mode shape-based forced vibration approach follows three main steps [35, 74]:

1. switching the interpolation basis from a Lagrange basis to a modal basis to reduce the number of degrees of freedom;
2. discretizing time using a periodic method, such as the spectral method (SM) or the harmonic quadrature method (HQM);
3. solving the discretized system for a different forcing frequency at the vicinity of its first eigen-frequency and plotting the frequency response curve.

##### 4.1.1 Switching the interpolation basis

As explained earlier, WQEM is a high-order FEM that relies on DQM to express the derivatives of the shape functions at the integration points [64]. This is technically, equivalent to making the following assumptions:

$$\begin{aligned}
\hat{w}(\xi, \tau) &= \underbrace{\begin{bmatrix} \mathcal{L}_1(\xi) \\ \vdots \\ \mathcal{L}_n(\xi) \end{bmatrix}}_{\mathcal{L}(\xi)} \cdot \underbrace{\begin{bmatrix} \hat{w}_1(\tau) \\ \vdots \\ \hat{w}_n(\tau) \end{bmatrix}}_{Y_1(\tau)} \quad (33) \\
\hat{\phi}(\xi, \tau) &= \mathcal{L}(\xi).Y_2(\tau)
\end{aligned}$$

where  $\mathcal{L}(\xi)$  is a vector of the Lagrange basis relative to  $\{\xi\}$  and the dimensions of each term are specified as a subscript in parentheses. Note that this discretization applies for both the displacements  $\hat{w}(\xi, \tau)$  and  $\hat{\phi}(\xi, \tau)$ , and virtual displacements  $\delta\hat{w}(\xi, \tau)$  and  $\delta\hat{\phi}(\xi, \tau)$ . To reduce the size of the problem,  $\hat{w}(\xi, \tau)$  and  $\hat{\phi}(\xi, \tau)$ , as well as  $\delta\hat{w}(\xi, \tau)$  and  $\delta\hat{\phi}(\xi, \tau)$ , are expressed using a reduced number of mode shapes  $m$ . Terms related to the reduced coordinates will use a double script font or will be underlined as indicated below

$$\hat{w}(\xi, \tau) = \underbrace{\begin{bmatrix} \mathcal{L}_1(\xi) \\ \vdots \\ \mathcal{L}_n(\xi) \end{bmatrix}}_{\mathcal{L}(\xi)} \cdot \underbrace{[\Phi_1]_{(n \times m)}}_{Y_1(\tau)} \cdot \underbrace{\begin{bmatrix} \underline{w}_1(\tau) \\ \vdots \\ \underline{w}_m(\tau) \end{bmatrix}}_{(m \times 1)} \quad (34a)$$

$$\hat{\phi}(\xi, \tau) = [\mathcal{L}(\xi)]^T \cdot [\Phi_2]_{(n \times m)} \cdot [\underline{Y}_2(\tau)] \quad (34b)$$

$$[\Phi_1]_{(n \times m)} = \underbrace{[\{\tilde{Y}_{w,1}\} \dots \{\tilde{Y}_{w,m}\}]}_{\text{The mode shape approximation basis for } \hat{w}(\xi, \tau)} \quad (34c)$$

$$[\Phi_2]_{(n \times m)} = \underbrace{[\{\tilde{Y}_{\phi,1}\} \dots \{\tilde{Y}_{\phi,m}\}]}_{\text{The mode shape approximation basis for } \hat{\phi}(\xi, \tau)} \quad (34d)$$

in which  $\underline{Y}_1(\tau)$  and  $\underline{Y}_2(\tau)$  denote, respectively, the reduced generalized coordinate vectors for  $\hat{w}(\xi, \tau)$  and  $\hat{\phi}(\xi, \tau)$ . The virtual displacements  $\delta\hat{w}(\xi, \tau)$  and  $\delta\hat{\phi}(\xi, \tau)$  are also interpolated in a similar manner.  $[\Phi_1]$  and

$[\Phi_2]$  are a collection of  $m$  columns representing linear eigenvectors relative to  $\hat{w}$  and  $\hat{\phi}$ , respectively, such that  $\{\tilde{Y}_{w,i}\}$  and  $\{\tilde{Y}_{\phi,i}\}$  are the  $i$ th linear eigenvectors with respect to  $\hat{w}$  and  $\hat{\phi}$ , respectively. The basic concept here is to interpolate the dependent variables and the virtual displacements using the limited number of dominant linear mode shapes. Hence, the reduced generalized coordinates in  $\mathbb{Y}_1(\tau)$  and  $\mathbb{Y}_2(\tau)$  are simply the amplitudes of each dominant linear mode.

Substituting into (25) gives a system of equations similar to Eqs. (32a) and (32b) as

$$[\omega_\xi \mathbb{M}_{1,0}]^T \cdot (\mathbb{M}_{1,0} \cdot \ddot{\mathbb{Y}}_1 + \hat{\mu}_f \mathbb{M}_{1,0} \cdot \dot{\mathbb{Y}}_1 - \hat{q}) + \hat{\mu}_0^2 [\omega_\xi \mathbb{M}_{1,1}]^T \cdot (\mathbb{M}_{1,1} \cdot \ddot{\mathbb{Y}}_1 + \hat{\mu}_f \mathbb{M}_{1,0} \cdot \dot{\mathbb{Y}}_1 - \hat{q}') + [\omega_\xi \mathbb{M}_{1,1}]^T \cdot \left( iC (\mathbb{M}_{1,1} \cdot \mathbb{Y}_1 - \hat{\mu}_0^2 \mathbb{M}_{1,3} \cdot \mathbb{Y}_1) + \frac{1}{s^2} (\mathbb{M}_{2,0} \cdot \mathbb{Y}_2 + \mathbb{M}_{1,1} \cdot \mathbb{Y}_1) \right) = 0 \quad (35a)$$

$$[\omega_\xi \mathbb{M}_{2,0}]^T \cdot \left( \frac{1}{s^2} (\mathbb{M}_{2,0} \cdot \mathbb{Y}_2 + \mathbb{M}_{1,1} \cdot \mathbb{Y}_1) + m \mathbb{M}_{2,0} \cdot \ddot{\mathbb{Y}}_2 + \hat{\mu}_0^2 (\mathbb{M}_{1,1} \cdot \dot{\mathbb{Y}}_1 + \hat{\mu}_f \mathbb{M}_{1,0} \cdot \dot{\mathbb{Y}}_1 - iC \mathbb{M}_{1,3} \cdot \mathbb{Y}_1) - \hat{q}' \right) + [\omega_\xi \mathbb{M}_{2,1}]^T \cdot (\mathbb{M}_{2,1} \cdot \mathbb{Y}_2 + \hat{\mu}_0^2 (m \mathbb{M}_{2,1} \cdot \ddot{\mathbb{Y}}_2 + \mathbb{M}_{1,0} \cdot \ddot{\mathbb{Y}}_1 + \hat{\mu}_f \mathbb{M}_{1,0} \cdot \dot{\mathbb{Y}}_1 - iC \mathbb{M}_{1,2} \cdot \mathbb{Y}_1 - \hat{q})) = 0 \quad (35b)$$

where

$$\begin{aligned} \mathbb{M}_{k,i} &= \hat{M}_i \cdot [\Phi_k] \\ iC &= \kappa_0 (\{\omega_\xi\} \cdot \{(\mathbb{M}_{1,1} \cdot \mathbb{Y}_1)^2\}) \\ \hat{q} &= \hat{k}_s \mathbb{M}_{1,2} \cdot \mathbb{Y}_1 - \hat{k}_L \mathbb{M}_{1,0} \cdot \mathbb{Y}_1 - \hat{k}_{NL} (\mathbb{M}_{1,0} \cdot \mathbb{Y}_1)^3 + \hat{\mathbb{F}} \cos(\hat{\omega} \tau) \\ \hat{q}' &= \hat{k}_s \mathbb{M}_{1,3} \cdot \mathbb{Y}_1 - \hat{k}_L \mathbb{M}_{1,1} \cdot \mathbb{Y}_1 - 3 \hat{k}_{NL} (\mathbb{M}_{1,0} \cdot \mathbb{Y}_1)^2 \mathbb{M}_{1,1} \cdot \mathbb{Y}_1 + \mathbb{M}_{1,0} \cdot \hat{\mathbb{F}} \cos(\hat{\omega} \tau) \end{aligned}$$

Switching to a mode shape interpolation basis reduces the problem size from  $2n$  equations ( $n$  is number of nodes in the mesh) to  $2m$ . This step is driven by the fact that a limited number of first mode shapes generally dominate the dynamic behavior of the nanobeam. In addition, the forcing term is focused to mostly excite the first  $w$  modes. Thus,  $\hat{F}$  or  $\hat{\mathbb{F}}$  is selected such that

$$\hat{F} = \mu_F \bar{F}_1 \{\tilde{Y}_{w,1}\} \quad (36a)$$

$$\hat{\mathbb{F}} = \mu_F \bar{F}_1 \{1 \ 0 \ \dots \ 0\}_{(m \times 1)}^T \quad (36b)$$

$$\mu_F = \left\{ \int_0^1 \psi^2 d\xi \right\}^{-1/2} = \|\psi\|_2^{-1} \approx \{\omega_\xi\} \cdot \{\tilde{Y}_{w,1}^2\}^{-1/2} \quad (36c)$$

where  $\bar{F}_1$  denotes the amplitude of the forcing term and  $\mu_F$  represents a scaling factor. Finally, let us define  $\mathbb{Y} = \begin{bmatrix} \mathbb{Y}_1 \\ \mathbb{Y}_2 \end{bmatrix}$ , so that Eqs. (35a) and (35b) can be expressed in the following form:

$$[M_{Sys}] \cdot \ddot{\mathbb{Y}} + [C_{Sys}] \cdot \dot{\mathbb{Y}} + [K_{Sys}(\mathbb{Y})] \cdot \mathbb{Y} - [F_{Sys}] \cos(\hat{\omega} \tau) = 0 \quad (37)$$

#### 4.1.2 Time discretization

Introducing a new timescale  $\hat{\tau}$  such that  $\hat{\tau} = \frac{\tau}{T} = \frac{\tau}{2\pi} \hat{\omega}$ , Eq. (37) becomes

$$\left( \frac{\hat{\omega}}{2\pi} \right)^2 [M_{Sys}] \cdot \ddot{\mathbb{Y}} + \left( \frac{\hat{\omega}}{2\pi} \right) [C_{Sys}] \cdot \dot{\mathbb{Y}} + [K_{Sys}(\mathbb{Y})] \cdot \mathbb{Y} - [F_{Sys}] \cos(2\pi \hat{\tau}) = 0 \quad (38)$$

The choice of this timescale eliminates the need to update the forcing term for various values of  $\hat{\omega}$ . A periodic steady-state solution must be reached to compute the frequency response of the system. This can be expressed with the following periodic initial conditions:

$$\begin{cases} \mathbb{Y}|_{\hat{\tau}=0} = \mathbb{Y}|_{\hat{\tau}=1} \\ \dot{\mathbb{Y}}|_{\hat{\tau}=0} = \dot{\mathbb{Y}}|_{\hat{\tau}=1} \end{cases} \quad (39)$$

A numerical solution for the forced vibration problem requires an adequate discretization of the time dimension. Both spectral method (SM) and harmonic differential quadrature method (HQM) can be utilized for this aim since both methods implicitly implement periodic initial conditions with additional accuracy of high-order methods. Hence, the corresponding compatible mesh is adopted

$$\hat{\tau}_i = \frac{i}{n_\tau}, \quad 0 < \hat{\tau}_i \leq 1, \quad 1 \leq i \leq n_\tau \quad n_\tau \text{ is even for SM} \quad (40)$$

$$\hat{\tau}_i = \frac{i}{n_\tau}, \quad 0 \leq \hat{\tau}_i < 1, \quad 0 \leq i \leq n_\tau - 1 \quad n_\tau \text{ is odd for HQM} \quad (41)$$

where  $n_\tau$  designates the time increment number. Here, the SM and HQM differentiation matrices are, respectively, provided in Appendices B and C and the discretized time space coordinate matrix is defined as

$$[Q] = \begin{bmatrix} [Q_w] \\ [Q_\phi] \end{bmatrix} = \begin{bmatrix} Y_1(\hat{\tau}_1) \cdots Y_1(\hat{\tau}_{n_\tau}) \\ Y_2(\hat{\tau}_1) \cdots Y_2(\hat{\tau}_{n_\tau}) \end{bmatrix} = \begin{bmatrix} \underline{w}_1(\hat{\tau}_1) \cdots \underline{w}_1(\hat{\tau}_{n_\tau}) \\ \vdots \\ \underline{w}_m(\hat{\tau}_1) \cdots \underline{w}_m(\hat{\tau}_{n_\tau}) \\ \underline{\phi}_1(\hat{\tau}_1) \cdots \underline{\phi}_n(\hat{\tau}_{n_\tau}) \\ \vdots \\ \underline{\phi}_m(\hat{\tau}_1) \cdots \underline{\phi}_m(\hat{\tau}_{n_\tau}) \end{bmatrix}_{(2m \times n_\tau)} \quad (42)$$

Here, the columns and lines of  $[Q]$  correspond to the discretized space and time, respectively. This approach yields the following discretized equation of motion:

$$\left(\frac{\hat{\omega}}{2\pi}\right)^2 [M_{Sys}].[Q].[D_\tau^{(2)}]^T + \left(\frac{\hat{\omega}}{2\pi}\right) [C_{Sys}].[Q].[D_\tau^{(1)}]^T + [K_{Sys}(Q)].[Q] - [F_{Sys}].[A] = 0 \quad (43)$$

in which  $[D_\tau^{(k)}]$  is the  $k$ th-order time derivative matrix and  $[A]$  is a  $(1 \times n_\tau)$  line matrix such as  $[A]_i = \cos(2\pi \hat{\tau}_i)$ . Finally, solving (43) for different values of  $\hat{\omega}$  in the neighborhood of the first linear frequency is required to obtain the frequency response curve.

#### 4.1.3 Frequency response curve

A frequency response curve can be generated in the neighborhood of each resonance. This applies to both system's dependent variables, namely  $\hat{w}$  and  $\hat{\phi}$ . The amplitudes of  $\hat{w}$ 's mode shapes as a function of time are obtained through the following transformation:

$$\underbrace{\begin{bmatrix} \underline{w}_1(\hat{\tau}) \\ \vdots \\ \underline{w}_m(\hat{\tau}) \end{bmatrix}}_{\{Y_1(\hat{\tau})\} \quad (m \times 1)} = \underbrace{\begin{bmatrix} \underline{w}_1(\hat{\tau}_1) \cdots \underline{w}_1(\hat{\tau}_{n_\tau}) \\ \vdots \\ \underline{w}_m(\hat{\tau}_1) \cdots \underline{w}_m(\hat{\tau}_{n_\tau}) \end{bmatrix}}_{[Q_w] \quad (m \times n_\tau)} \cdot \underbrace{\begin{bmatrix} \mathcal{L}_1(\hat{\tau}) \\ \vdots \\ \mathcal{L}_{n_\tau}(\hat{\tau}) \end{bmatrix}}_{\{\mathcal{L}(\hat{\tau})\} \quad (n_\tau \times 1)} \quad (44)$$

where  $[Q_w]$  is defined in (42). Finally,  $\{\mathcal{L}(\tau)\}$ , the time discretization basis is given by [76–78]

$$\begin{aligned} \mathcal{L}_i(\tau) &= \frac{1}{n_\tau} \cot\left(\pi\left(t - \frac{i}{n_\tau}\right)\right) \sin\left(\pi n_\tau\left(t - \frac{i}{n_\tau}\right)\right) \quad \text{for spectral method} \\ \mathcal{L}_i(\tau) &= \prod_{j=1, j \neq i}^{n_\tau} \frac{\sin(\pi(\tau - \tau_j))}{\sin(\pi(\tau_i - \tau_j))} \quad \text{for HQM} \end{aligned} \quad (45)$$

The frequency response curve was established such that the forcing term excited only the fundamental natural frequency. This corresponds to finding  $\max(\underline{w}_1(\tau))$  of the QEM system (32) at several values of  $\hat{\omega}$  in the neighborhood of the first natural frequency.

## 4.2 WQEM formulation using Galerkin technique

It is also possible to write (32a) and (32b) as

$$[M_{Sys}].\ddot{Y} + [C_{Sys}].\dot{Y} + [K_{Sys}(Y)].Y - [F_{Sys}] \cos(\hat{\omega}\tau) = 0 \quad (46)$$

Based on this form, the forced vibration problem can be treated similarly to the DQM case in [74, 75, 79, 80]. Despite the similarity, there are few differences to be noted

1. In this case, a Galerkin projection is applied to discretize the variational statement.
2. Two nested integrals have to be calculated: the WQEM integral and the Galerkin integral.
3. Thanks to the careful mesh choice, it is possible to use the same high-order integration scheme for the WQEM and Galerkin integrals.
4. This method is computationally more intensive than the method presented in the previous section.
5. The main difference between this approach and the one presented in the previous section is the procedure of reducing the number of the degrees of freedom.

Going back to (46), all matrices present in this equation along with  $[F_{Sys}]$  are established using WQEM, i.e., the high-order variational statement. Now that the system looks like a time-dependent differential equation, the Galerkin technique is adopted to limit the size of the WQEM system to  $2m$  equations. As stated in the previous section, only a limited number of mode shapes dominate the nanobeam's vibrational response. Hence, the  $2m$  first linear nonlocal mode shapes are selected as the basis in applying the Galerkin technique [74, 75, 79, 80]. Consequently, the following change of variables has to be made:

$$\underbrace{\begin{bmatrix} Y_1 \\ Y_2 \end{bmatrix}}_Y \quad (2n \times 1) = \underbrace{\begin{bmatrix} [\Phi_w] & 0 \\ 0 & [\Phi_\phi] \end{bmatrix}}_{[\Phi]} \quad (2n \times 2m) \cdot \underbrace{\begin{bmatrix} \{\mathcal{Y}_1(\tau)\}_{(m \times 1)} \\ \{\mathcal{Y}_2(\tau)\}_{(m \times 1)} \end{bmatrix}}_{\mathcal{Y}(\tau)} \quad (2m \times 1) \quad (47)$$

where  $\mathcal{Y}(\tau)$  and  $[\Phi]$  are, respectively, the reduced generalized coordinates and the Galerkin approximation basis. The idea is similar to the one presented in the previous section except here the change of variable is performed after the evaluation of the variational statement.  $\{\mathcal{Y}_1(\tau)\}$  and  $\{\mathcal{Y}_2(\tau)\}$  denote, respectively, the reduced generalized coordinates relative to  $\hat{w}$  and  $\hat{\phi}$ .  $[\Phi]$  is composed of two blocks  $[\Phi_w]$  and  $[\Phi_\phi]$ , which are identical to the ones presented in the previous section. The Galerkin approximation consists of premultiplying Eq. (46) by a numerical Galerkin projection operator denoted by  $[G]$  to yield the following:

$$\underbrace{[[S][\Phi]]^T}_{[G]_{(2m \times 2n)}} \cdot ([M_{Sys}].[\Phi].\ddot{\mathcal{Y}} + [C_{Sys}].[\Phi].\dot{\mathcal{Y}} + [K_{Sys}([\Phi].\mathcal{Y})].[\Phi].\mathcal{Y} - [F_{Sys}] \cos(\hat{\omega}\tau)) = 0 \quad (48)$$

in which  $[S]$  is the integral quadrature weight coefficient matrix and  $[\Phi]$  is given by (47). In the literature,  $[S]$  is usually computed using the trapezoidal rule [36, 65, 75]. However, since a GLL grid is used, a more accurate result can be obtained by setting  $[S] = [\omega_\xi]$ . The final system is given by

$$\overline{[M_{Sys}]}.\ddot{\mathcal{Y}} + \overline{[C_{Sys}]}.\dot{\mathcal{Y}} + \overline{[K_{Sys}(\mathcal{Y})]}.\mathcal{Y} - \overline{[F_{Sys}]} \cos(\hat{\omega}\tau) = 0 \quad (49)$$

where

$$\begin{aligned} \overline{[M_{Sys}]}_{(2m \times 2m)} &= [G]_{(2m \times 2n)} \cdot [M_{Sys}]_{(2n \times 2n)} \cdot [\Phi]_{(2n \times 2m)} \\ \overline{[K_{Sys}(\mathcal{Y}(\tau))]}_{(2m \times 2m)} &= [G]_{(2m \times 2n)} \cdot [K_{Sys}([\Phi].\mathcal{Y}(\tau))]_{(2n \times 2n)} \cdot [\Phi]_{(2n \times 2m)} \\ \overline{[C_{Sys}]}_{(2m \times 2m)} &= [G]_{(2m \times 2n)} \cdot [C_{Sys}]_{(2n \times 2n)} \cdot [\Phi]_{(2n \times 2m)} \\ \overline{[F_{Sys}]}_{(2m \times 1)} &= [G]_{(2m \times 2n)} \cdot [F_{Sys}]_{(2n \times 1)} \end{aligned}$$

This system looks similar to (37), although the resulting matrices are not the same. Nevertheless, the solution procedure is exactly the same from this point onward.

## 5 Numerical results and discussion

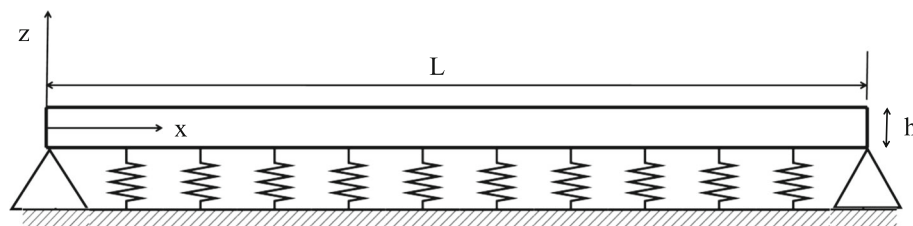
Various aspects of the graded nanobeam vibration are presented herein including nonlocal linear and nonlinear frequencies in addition to force vibration frequency response curves. A schematic of the beam is presented in Fig. 1. It is assumed that the beam has a square cross section such that  $b = h = \frac{1}{10}L$ . It is further assumed that its material distribution is graded in the  $z$ -direction according to Eq. (11) where  $-\frac{h}{2} \leq z \leq \frac{h}{2}$  and  $P_L$  and  $P_U$  designate aluminum and silicon properties, respectively. In this study, the utilized material properties are reported in Table 1 and the considered boundary conditions include hinged–hinged (HH) and clamped–clamped (CC).

### 5.1 Performance of WQEM

A mesh convergence study is the first step. To give a better assessment of the convergence performance of WQEM, it was decided to conduct a mesh convergence study on all cases treated later in Table 5. Generally, the CC case is the slowest to converge. A selection of these difficult CC cases are presented in Table 2 along with their HH analogs. Table 2 shows that the errors for the CC cases fall below 0.5% for as few as 7 nodes. A choice of 7 nodes is totally acceptable, although an 11-node grid is selected for better accuracy. In fact, the error for the HH cases drops below 0.05% using just 7 nodes. This rapid convergence is one of the major advantages for using a high-order variational statement. A similar system would have required 15 nodes to converge using DQM [36]. It is important to note that these results are obtained despite the fact that the nanobeam is highly nonlinear. In this study, a choice of 11 nodes is adopted. The DQM results in this paper were provided from a study by Trabelssi et al. [36] which used a 15-node grid.

### 5.2 Free vibration response

The aim of this section is to replicate the results obtained by Trabelssi et al. [36] for a similar problem using WQEM. The results obtained herein can be divided into two tables. First, the effect of different parameters including the material inhomogeneity index  $n_k$ , the amplitude of the free vibration  $A$  and the stiffness parameters of the elastic foundation, on the free vibration of the nanobeam is investigated in Table 3 for HH and CC nanobeams. The results reported in Table 3 were generated based on the WQEM discretization. For the sake of comparison, Table 3 contains DQM data obtained by Trabelssi et al. [36], which helps to assess the performance of the proposed approach. The present data utilize a range of values of the amplitude and the nonlocal parameter  $A$  and  $\hat{\mu}_0$ , while the inhomogeneity index  $n_k$  varies from 1 to 4. Both HH and CC configurations are included in this study, while  $L/h$  varies between 10 to 100. The foundation stiffness configuration is described with the following parameters  $\hat{k}_s = 5$ ,  $\hat{k}_L = 50$  and  $\hat{k}_{NL} = 50$ . Table 3 shows that the frequencies obtained using WQEM match their DQM counterparts up to the fourth digit regardless of the configuration of the



**Fig. 1** A hinged–hinged nanobeam resting on a nonlinear elastic foundation

**Table 1** Material properties of the constituent materials of the graded nanobeam [81]

Material		Young modulus (GPa)	Poisson's ratio	Density (kg/m <sup>3</sup> )
Metal:	Aluminum	70	0.24	2700
Ceramic:	Silicon	210	0.3	2370

**Table 2** WQEM convergence performance ( $\frac{l}{h} = 10, \hat{\mu}_0^2 = 5, n_k = 0.5, A = 1$ ): slowest converging cases selected from the study performed in Table 5

$\hat{k}_L$	$\hat{k}_{NL}$	$\hat{k}_S$	WQEM frequencies					WQEM Error to the finest grid ( $n_x = 15$ )					
			$n_x$	5	7	9	11	13	15	5	7	9	11
CC boundary condition													
0	10	0	18.9494	18.8515	18.8457	18.8460	18.8459	18.8460	0.55%	0.03%	0.00%	0.00%	0.00%
			50	39.1014	38.2234	38.3300	38.3410	38.3300	38.3349	2.00%	0.29%	0.01%	0.02%
	50	0	19.9220	19.6928	19.6725	19.6747	19.6744	19.6745	1.26%	0.09%	0.01%	0.00%	0.00%
			50	39.5717	38.6928	38.6940	38.7806	38.7275	38.7479	2.13%	0.14%	0.14%	0.08%
	100	0	21.0748	20.7009	20.6692	20.6742	20.6739	20.6739	1.94%	0.13%	0.02%	0.00%	0.00%
			50	40.1518	39.2720	39.1479	39.3220	39.2238	39.2559	2.28%	0.04%	0.28%	0.17%
50	10	0	20.2043	20.1123	20.1069	20.1072	20.1071	20.1071	0.48%	0.03%	0.00%	0.00%	0.00%
			50	39.7251	38.8613	38.9661	38.9769	38.9661	38.9709	1.94%	0.28%	0.01%	0.02%
	50	0	21.1192	20.9029	20.8838	20.8859	20.8856	20.8857	1.12%	0.08%	0.01%	0.00%	0.00%
			50	40.1882	39.3230	39.3242	39.4094	39.3571	39.3773	2.06%	0.14%	0.13%	0.08%
	100	0	22.2099	21.8553	21.8253	21.8300	21.8297	21.8297	1.74%	0.12%	0.02%	0.00%	0.00%
			50	40.7595	39.8931	39.7710	39.9422	39.8457	39.8772	2.21%	0.04%	0.27%	0.16%
HH boundary condition													
0	10	0	9.8477	9.8282	9.8283	9.8283	9.8283	9.8283	0.20%	0.00%	0.00%	0.00%	0.00%
			50	24.2162	24.2082	24.2080	24.2081	24.2081	24.2081	0.03%	0.00%	0.00%	0.00%
	50	0	11.3241	11.2678	11.2698	11.2699	11.2699	11.2699	0.48%	0.02%	0.00%	0.00%	0.00%
			50	24.8499	24.8191	24.8185	24.8187	24.8187	24.8187	0.13%	0.00%	0.00%	0.00%
	100	0	12.9362	12.8625	12.8692	12.8694	12.8693	12.8693	0.52%	0.05%	0.00%	0.00%	0.00%
			50	25.6200	25.5649	25.5642	25.5645	25.5645	25.5645	0.22%	0.00%	0.00%	0.00%
10	0	0	12.1068	12.0910	12.0910	12.0911	12.0911	12.0911	0.13%	0.00%	0.00%	0.00%	0.00%
			50	25.2195	25.2118	25.2116	25.2117	25.2117	25.2117	0.03%	0.00%	0.00%	0.00%
	50	0	13.3354	13.2876	13.2894	13.2895	13.2895	13.2895	0.35%	0.01%	0.00%	0.00%	0.00%
			50	25.8285	25.7990	25.7984	25.7985	25.7985	25.7985	0.12%	0.00%	0.00%	0.00%
	100	0	14.7290	14.6644	14.6703	14.6705	14.6704	14.6704	0.40%	0.04%	0.00%	0.00%	0.00%
			50	26.5703	26.5172	26.5165	26.5168	26.5168	26.5168	0.20%	0.00%	0.00%	0.00%

nanobeam, the vibration amplitude or the boundary conditions. Knowing that WQEM results were obtained with a significantly lower mesh density, this truly assesses the accuracy of the WQEM data.

To assess the sensitivity of the proposed formulation to shear locking, the same configuration used to generate the data in Table 3 is used to recompute the nonlocal nonlinear frequencies for thick nanobeams where the aspect ratio  $\frac{l}{h}$  is kept below 10. To the best of the authors' knowledge, there has been no study that confirmed the presence of shear locking in DQM. In light of this, DQM data were also generated to assess the accuracy of the WQEM results. According to Table 4, the data generated using WQEM are in good agreement with DQM data, indicating the absence of shear locking. This is in agreement with Jin and Wang [82] who also found no shear locking in their linear classical WQEM TBT model.

The effect of the nonlinear foundation is investigated in Table 5. Using the same set of boundary conditions, the nonlinear nonlocal frequencies were computed using WQEM along with DQM data [36]. The values of the different parameters are chosen to underline the effect of the nonlinear elastic foundation which is the highest nonlinear element in the system. It is also the only controllable nonlinearity in the system. The foundation's linear and nonlinear coefficients  $\hat{k}_L$  and  $\hat{k}_{NL}$  are set to vary between 0 and 100 for the former and between 10 and 100 for the latter. The shear coefficient  $\hat{k}_S$  varies between 0 and 50, while the inhomogeneity index  $n_k$  and the amplitude  $A$  are set to a fixed value of 0.5 and 1, respectively. Both  $\hat{k}_S$  and  $\hat{k}_{NL}$  are known to affect considerably the behavior of the nanobeam [36]. Despite the wide range of the selected variables, Table 5 shows that the WQEM and DQM results still show great consistency regardless of the selected configuration. In fact, the WQEM low density mesh still performs as good as DQM's higher-density mesh. Technically, variational methods converge faster than their collocation counterparts indicating that these results were expected. The use of the GLL grid helped improve the integration accuracy although the mass matrix is still not fully integrated. It is still possible to improve the accuracy of WQEM with a higher-order integration technique, although this may increase the complexity of the implementation [37].



**Table 3** Effect of the material inhomogeneity index  $n_k$  on the nonlinear nonlocal frequencies of a FG TBT nanobeam ( $\hat{k}_s = 5$ ;  $\hat{k}_L = 50$  and  $\hat{k}_{NL} = 50$ )

$\frac{L}{h}$	A	$\hat{\mu}_0^2$	DQM [36]				WQEM				
			$n_k$	1	2	3	4	1	2	3	4
HH boundary condition											
10	0	0	13.9369	13.9348	13.9333	13.9324	13.9369	13.9348	13.9333	13.9324	
		1	13.6251	13.6231	13.6218	13.6209	13.6251	13.6231	13.6218	13.6209	
		3	13.1293	13.1277	13.1265	13.1258	13.1293	13.1277	13.1265	13.1258	
	0.5	0	14.4771	14.4596	14.4491	14.4435	14.4771	14.4596	14.4491	14.4435	
		1	14.1774	14.1597	14.1491	14.1435	14.1774	14.1597	14.1491	14.1435	
		3	13.7021	13.6842	13.6734	13.6677	13.7021	13.6842	13.6734	13.6677	
	1	0	15.9917	15.9338	15.8994	15.8814	15.9917	15.9338	15.8994	15.8814	
		1	15.7242	15.6653	15.6304	15.6121	15.7242	15.6653	15.6304	15.6121	
		3	15.3027	15.2424	15.2066	15.1878	15.3027	15.2424	15.2066	15.1878	
	100	0	0	14.0261	14.0261	14.0261	14.0260	14.0261	14.0261	14.0261	14.0260
			1	13.7107	13.7107	13.7106	13.7106	13.7107	13.7107	13.7106	13.7106
			3	13.2092	13.2092	13.2092	13.2091	13.2092	13.2092	13.2092	13.2091
0.5		0	14.5671	14.5516	14.5425	14.5377	14.5671	14.5516	14.5425	14.5377	
		1	14.2639	14.2481	14.2387	14.2339	14.2639	14.2481	14.2387	14.2339	
		3	13.7830	13.7666	13.7569	13.7519	13.7830	13.7666	13.7569	13.7519	
1		0	16.0844	16.0280	15.9948	15.9775	16.0844	16.0280	15.9948	15.9775	
		1	15.8133	15.7559	15.7220	15.7043	15.8133	15.7559	15.7220	15.7043	
		3	15.3863	15.3271	15.2921	15.2739	15.3863	15.3271	15.2921	15.2739	
CC boundary condition											
10		0	0	24.1208	24.0840	24.0594	24.0456	24.1208	24.0840	24.0594	24.0456
			1	23.3367	23.3036	23.2814	23.2689	23.3367	23.3036	23.2814	23.2689
	3		22.1324	22.1048	22.0863	22.0757	22.1324	22.1048	22.0863	22.0757	
	0.5	0	24.4560	24.4086	24.3778	24.3607	24.4560	24.4086	24.3778	24.3607	
		1	23.7261	23.6791	23.6487	23.6320	23.7261	23.6791	23.6487	23.6320	
		3	22.6093	22.5626	22.5329	22.5165	22.6093	22.5626	22.5329	22.5165	
	1	0	25.4351	25.3575	25.3090	25.2827	25.4351	25.3575	25.3090	25.2827	
		1	24.8580	24.7719	24.7185	24.6898	24.8581	24.7719	24.7185	24.6898	
		3	23.9850	23.8856	23.8247	23.7921	23.9850	23.8856	23.8247	23.7921	
	100	0	0	24.7300	24.7296	24.7293	24.7291	24.7300	24.7296	24.7293	24.7291
			1	23.9267	23.9263	23.9261	23.9259	23.9267	23.9263	23.9261	23.9259
			3	22.6875	22.6872	22.6870	22.6869	22.6875	22.6872	22.6870	22.6869
0.5		0	25.0640	25.0526	25.0459	25.0423	25.0640	25.0526	25.0459	25.0423	
		1	24.3167	24.3020	24.2933	24.2888	24.3167	24.3020	24.2933	24.2888	
		3	23.1672	23.1473	23.1356	23.1295	23.1672	23.1473	23.1356	23.1295	
1		0	26.0404	25.9977	25.9725	25.9593	26.0404	25.9977	25.9725	25.9593	
		1	25.4515	25.3964	25.3639	25.3470	25.4515	25.3964	25.3639	25.3470	
		3	24.5520	24.4779	24.4342	24.4114	24.5520	24.4779	24.4342	24.4114	

### 5.3 Forced vibration response

The forced vibration of the nanobeam is examined in this section. This investigation covers both WQEM force vibration approaches detailed in Sects. 4.1 and 4.2 in addition to the previously validated DQM [36]. For each method, the FRC is plotted individually for various values of  $\hat{k}_s$  and for  $n_k = 2$ ,  $\hat{\mu}_0^2 = 2$ ,  $\hat{k}_{NL} = 50$ ,  $\hat{k}_L = 10$ ,  $\bar{F}_1 = 0.75$  and  $\frac{L}{h} = 10$ . This configuration is chosen due to its highly nonlinear behavior and its high dependency on  $\hat{k}_s$ . All WQEM FRCs were generated using only 9 nodes, while the DQM FRCs were generated based on a 15-node grid. Figure 2 shows all of the FRC plots for each method as well as a plot of all methods together. These plots are generated for the HH nanobeam. These results show that, despite the lower mesh density, both WQEM methods were able to achieve the same level of convergence as the 15-node DQM results. In fact, Fig. 2 shows that the FRCs totally overlap. The same FRCs are plotted for the CC case in Fig. 3. For this configuration,  $\bar{F}_1$  had to be raised to  $\bar{F}_1 = 1.5$  in order to obtain a comparable deformation to the HH case. The mesh density is left unchanged and the FRCs are plotted in a similar manner. Figure 3 shows similar convergence of all methods since again all of the FRCs overlap. Based on the reported results, it is clear that WQEM offers a significant computational advantage over DQM. The method requires fewer nodes than DQM, and WQEM solves this FEM problem using a single high-order element without the need to explicitly identify the shape functions. This simplifies its implementation and eliminates the need of element

**Table 4** Effect of the material inhomogeneity index  $n_k$  on the nonlinear nonlocal frequencies of a FG TBT thick nanobeam ( $\hat{k}_s = 5$ ;  $\hat{k}_L = 50$  and  $\hat{k}_{NL} = 50$ )

$\frac{L}{h}$	A	$\hat{\mu}_0^2$	DQM*				WQEM			
			$n_k$	1	2	3	4	1	2	3
HH boundary condition										
4	0.5	0	14.0539	14.0294	14.0137	14.0052	14.0539	14.0294	14.0137	14.0052
		1	13.7709	13.7470	13.7318	13.7235	13.7709	13.7470	13.7318	13.7235
		3	13.3224	13.2994	13.2849	13.2769	13.3225	13.2995	13.2850	13.2771
	1	0	15.5572	15.4943	15.4561	15.4358	15.5572	15.4943	15.4561	15.4358
		1	15.3063	15.2433	15.2051	15.1848	15.3063	15.2433	15.2051	15.1848
		3	14.9102	14.8469	14.8085	14.7882	14.9111	14.8479	14.8097	14.7894
5	0.5	0	14.2251	14.2031	14.1892	14.1817	14.2251	14.2031	14.1892	14.1817
		1	13.9353	13.9136	13.8999	13.8925	13.9353	13.9136	13.8999	13.8925
		3	13.4760	13.4546	13.4414	13.4342	13.4760	13.4547	13.4414	13.4342
	1	0	15.7328	15.6715	15.6345	15.6149	15.7328	15.6715	15.6345	15.6149
		1	15.4752	15.4134	15.3763	15.3566	15.4752	15.4134	15.3763	15.3566
		3	15.0691	15.0066	14.9691	14.9492	15.0694	15.0069	14.9694	14.9496
8	0.5	0	14.4276	14.4091	14.3978	14.3918	14.4276	14.4091	14.3978	14.3918
		1	14.1298	14.1112	14.1000	14.0939	14.1298	14.1112	14.1000	14.0939
		3	13.6576	13.6389	13.6276	13.6216	13.6576	13.6389	13.6276	13.6216
	1	0	15.9408	15.8821	15.8471	15.8287	15.9408	15.8821	15.8471	15.8287
		1	15.6751	15.6156	15.5802	15.5616	15.6751	15.6156	15.5802	15.5616
		3	15.2567	15.1959	15.1597	15.1407	15.2567	15.1959	15.1597	15.1407
CC boundary condition										
4	0.5	0	22.0435	21.8872	21.7853	21.7288	22.0435	21.8872	21.7853	21.7288
		1	21.4002	21.2564	21.1625	21.1104	21.4003	21.2566	21.1627	21.1106
		3	20.4413	20.3165	20.2349	20.1896	20.4440	20.3197	20.2385	20.1933
	1	0	23.0556	22.8748	22.7587	22.6950	23.0556	22.8748	22.7587	22.6950
		1	22.5422	22.3654	22.2523	22.1904	22.5432	22.3664	22.2535	22.1915
		3	21.7944	21.6215	21.5115	21.4513	21.8196	21.6510	21.5440	21.4857
5	0.5	0	22.9436	22.8219	22.7422	22.6979	22.9436	22.8219	22.7422	22.6979
		1	22.2651	22.1519	22.0778	22.0367	22.2652	22.1520	22.0779	22.0368
		3	21.2429	21.1425	21.0769	21.0405	21.2440	21.1438	21.0783	21.0420
	1	0	23.9391	23.7905	23.6952	23.6429	23.9390	23.7905	23.6952	23.6429
		1	23.3994	23.2506	23.1558	23.1040	23.3998	23.2511	23.1564	23.1045
		3	22.6038	22.4543	22.3599	22.3084	22.6124	22.4640	22.3704	22.3194
8	0.5	0	24.1381	24.0733	24.0309	24.0074	24.1381	24.0733	24.0309	24.0074
		1	23.4180	23.3555	23.3148	23.2922	23.4181	23.3555	23.3148	23.2922
		3	22.3196	22.2602	22.2219	22.2008	22.3199	22.2605	22.2222	22.2010
	1	0	25.1194	25.0251	24.9654	24.9329	25.1194	25.0251	24.9654	24.9329
		1	24.5494	24.4484	24.3851	24.3508	24.5495	24.4485	24.3852	24.3509
		3	23.6914	23.5800	23.5111	23.4739	23.6929	23.5815	23.5127	23.4756

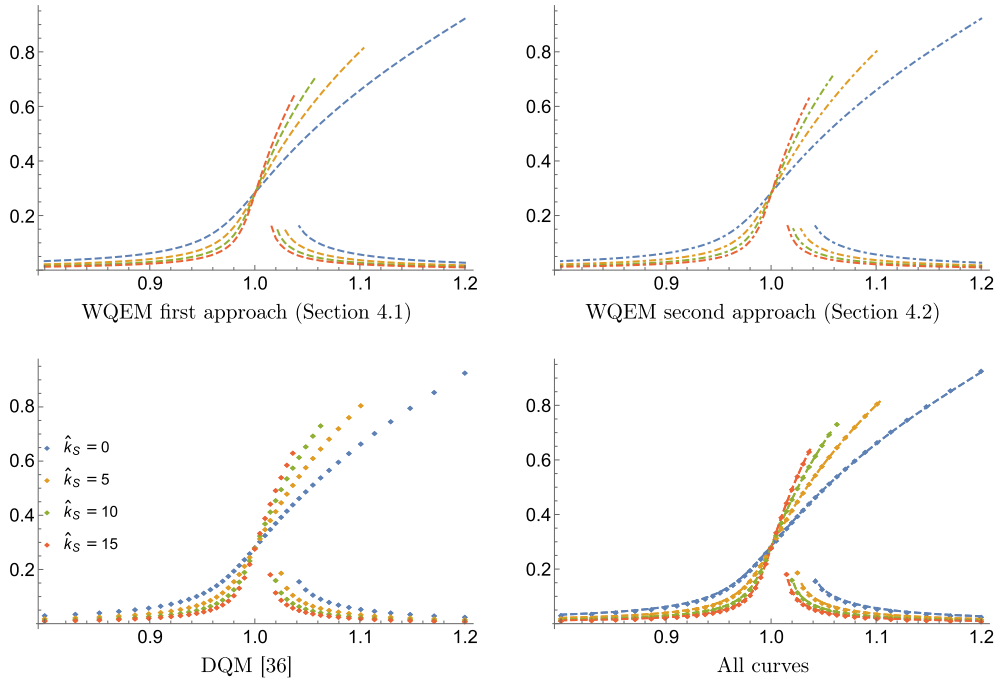
\*Results computed based on the DQM formulation by Trabelssi et al [36]

assembly. It is important to note that the first approach requires less computational effort than the second one since only one integration is required, while the second approach requires two integrations.

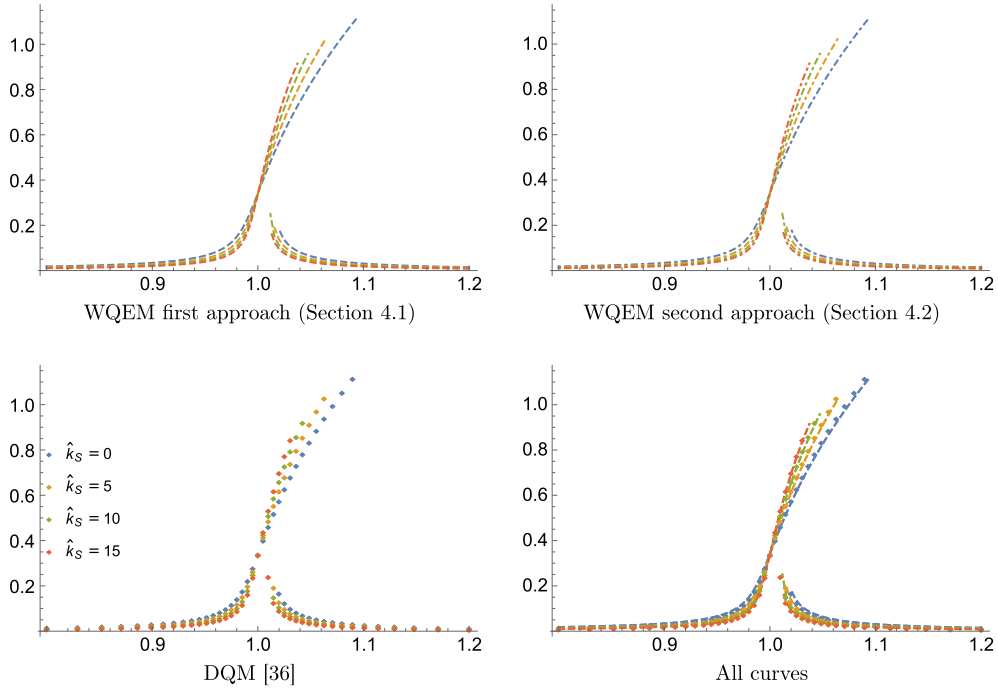
## 6 Conclusion

A general formulation of the WQEM with arbitrary element order is presented for an FG nonlocal nonlinear nanobeam based on Timoshenko beam theory. Eringen's nonlocal elasticity was employed to capture size effects of the nanobeam, and a power-law function was utilized to model the material property distribution in the nanobeam. The use of DQ rule simplifies the implementation of the proposed WQEM elements and easily allows an increase in the element order, while the GLL mesh guarantees a significant integration accuracy. For the sake of generality, the formulation accounts for the nonlinear von Kármán strain as well as the contribution of the nonlinear elastic foundation.

The suitability and computational efficiency of the proposed quadrature elements for the vibration analysis of FG beams are demonstrated. A free vibration study was carried out for several mesh densities as well as a set of nonlinear configurations. The study shows an improved convergence rate compared to DQM. The free vibration results indicate that the proposed quadrature FG Timoshenko nonlinear nanobeam element is



**Fig. 2** Frequency response curves for the HH TBT: effect of  $\hat{k}_s$  [ $n_k = 2, \hat{\mu}_0^2 = 2, \hat{k}_{NL} = 50, \hat{k}_L = 10, \bar{F}_1 = 0.75$ ]



**Fig. 3** Frequency response curves for the CC beam: effect of  $\hat{k}_s$  [ $n_k = 2, \hat{\mu}_0^2 = 2, \hat{k}_{NL} = 50, \hat{k}_L = 10, \bar{F}_1 = 1.5$ ]

**Table 5** Effect of the nonlinear elastic foundation ( $A = 1; n_k = 0.5$ )

$\frac{L}{h}$	$\hat{k}_L$	$\hat{\mu}_0^2$	DQM [36]					WQEM						
			$\hat{k}_S$		0		50		0		50			
			10	50	100	10	50	100	10	50	100	10	50	100
HH boundary condition														
10	0	0	11.3200	12.5710	13.9823	24.8524	25.4457	26.1699	11.3200	12.5710	13.9823	24.8524	25.4457	26.1699
		1	10.9344	12.2295	13.6849	24.6790	25.2770	26.0073	10.9344	12.2295	13.6849	24.6790	25.2771	26.0074
		5	9.8283	11.2699	12.8693	24.2081	24.8194	25.5671	9.8283	11.2699	12.8693	24.2081	24.8187	25.5645
	50	0	13.3319	14.4093	15.6558	25.8310	26.4023	27.1009	13.3319	14.4093	15.6558	25.8310	26.4023	27.1009
		1	13.0061	14.1124	15.3908	25.6641	26.2398	26.9440	13.0061	14.1124	15.3908	25.6641	26.2398	26.9441
		5	12.0911	13.2895	14.6704	25.2117	25.7992	26.5193	12.0911	13.2895	14.6704	25.2117	25.7985	26.5168
	100	0	15.0177	16.0383	17.1669	26.7738	27.3255	28.0010	15.0177	16.0383	17.1669	26.7738	27.3255	28.0010
		1	14.7904	15.7720	16.9255	26.6129	27.1684	27.8492	14.7904	15.7720	16.9255	26.6129	27.1684	27.8492
		5	13.9926	15.0403	16.2734	26.1768	26.7432	27.4384	13.9926	15.0403	16.2734	26.1768	26.7425	27.4360
	100	0	11.4053	12.6570	14.0696	24.9703	25.5655	26.2918	11.4053	12.6570	14.0696	24.9703	25.5655	26.2918
		1	11.0158	12.3114	13.7676	24.7946	25.3946	26.1272	11.0158	12.3114	13.7676	24.7946	25.3946	26.1272
		5	9.8982	11.3395	12.9381	24.3177	24.9303	25.6785	9.8982	11.3395	12.9381	24.3177	24.9303	25.6785
	50	0	13.4193	14.4981	15.7464	25.9521	26.5252	27.2260	13.4193	14.4981	15.7464	25.9521	26.5252	27.2260
		1	13.0899	14.1974	15.4772	25.7831	26.3606	27.0671	13.0899	14.1974	15.4772	25.7831	26.3606	27.0671
		5	12.1643	13.3634	14.7442	25.3248	25.9136	26.6342	12.1643	13.3634	14.7442	25.3248	25.9136	26.6342
	100	0	15.1682	16.1304	17.2611	26.8981	27.4515	28.1292	15.1682	16.1304	17.2611	26.8981	27.4515	28.1292
		1	14.8775	15.8607	17.0159	26.7351	27.2924	27.9754	14.8775	15.8607	17.0159	26.7351	27.2924	27.9754
		5	14.0701	15.1187	16.3520	26.2933	26.8610	27.5568	14.0701	15.1187	16.3520	26.2933	26.8610	27.5568
CC boundary condition														
10	0	0	22.5928	23.2288	24.0011	33.0229	33.4639	34.0078	22.5928	23.2288	24.0011	33.0229	33.4639	34.0078
		1	21.5853	22.2663	23.0918	34.6531	35.0810	35.6096	21.5853	22.2663	23.0918	34.6531	35.0810	35.6096
		5	18.8460	19.6745	20.6739	38.3348	38.7472	39.2540	18.8460	19.6745	20.6739	38.3349	38.7479	39.2559
	50	0	23.6637	24.2716	25.0118	33.7649	34.1964	34.7288	23.6637	24.2716	25.0118	33.7649	34.1964	34.7288
		1	22.7012	23.3496	24.1381	35.3593	35.7787	36.2971	22.7012	23.3496	24.1381	35.3593	35.7787	36.2971
		5	20.1071	20.8857	21.8297	38.9708	39.3766	39.8753	20.1071	20.8857	21.8297	38.9709	39.3773	39.8772
	100	0	24.6882	25.2715	25.9832	34.4911	34.9136	35.4352	24.6882	25.2715	25.9832	34.4911	34.9136	35.4352
		1	23.7647	24.3848	25.1409	36.0516	36.4631	36.9719	23.7647	24.3848	25.1409	36.0516	36.4631	36.9719
		5	21.2937	22.0304	22.9274	39.5966	39.9960	40.4872	21.2937	22.0304	22.9274	39.5967	39.9967	40.4890
	100	0	23.2071	23.8323	24.5923	33.7209	34.1567	34.6943	23.2071	23.8323	24.5923	33.7209	34.1567	34.6943
		1	22.1771	22.8480	23.6622	35.4379	35.8614	36.3848	22.1771	22.8480	23.6622	35.4379	35.8614	36.3848
		5	19.3667	20.1872	21.1769	39.1219	39.5310	40.0378	19.3667	20.1872	21.1769	39.1219	39.5310	40.0378
	50	0	24.2603	24.8590	25.5886	34.4542	34.8808	35.4075	24.2603	24.8590	25.5886	34.4542	34.8808	35.4075
		1	23.2769	23.9171	24.6960	36.1364	36.5518	37.0654	23.2769	23.9171	24.6960	36.1364	36.5518	37.0654
		5	20.6170	21.3895	22.3260	39.7557	40.1583	40.6573	20.6170	21.3895	22.3260	39.7557	40.1583	40.6573
	100	0	25.2697	25.8451	26.5476	35.1723	35.5903	36.1066	25.2697	25.8451	26.5476	35.1723	35.5903	36.1066
		1	24.3271	24.9403	25.6883	36.8216	37.2294	37.7338	24.3271	24.9403	25.6883	36.8216	37.2294	37.7338
		5	21.7957	22.5278	23.4188	40.3795	40.7760	41.2675	21.7957	22.5278	23.4188	40.3795	40.7760	41.2675

highly accurate and efficient. A study of the nonlinear free vibration of thick nanobeams revealed that the proposed element is shear locking free and can yield accurate solutions with a small number of nodes for both thin and moderately thick nanobeams. This approach offers the precision of high-order methods such as DQM without sacrificing the flexibility of variational methods such as FEM. In addition, due to the presence of von Kármán strain and the nonlinear foundation, high-order derivatives are required to evaluate accurately the system response. Such behavior is hard to capture using conventional low-order methods and high-order collocation methods can be used to overcome this problem. WQEM solves this by offering high-order accuracy in a variational method.

The study also aims to establish a standard procedure to solve forced vibration WQEM problems. In general, plotting FRCs using numerical methods is challenging due to limitations related to time integration and transient response. Various types of periodic time discretization are generally used which allows time to be treated similarly to a space dimension. However, it consequently adds an extra dimension to the computational problem. Generally DQM force vibration systems resort to Galerkin techniques to reduce the computational cost. To achieve a similar goal for WQEM problems, the authors proposed two different methods to reduce the number degrees of freedom of the WQEM system. The first approach replaces the Lagrange interpolation shape functions generally used in WQEM with mode shape-based functions. The second method utilizes a similar approach to the one used in DQM. Both methods were validated based on a previous DQM study performed by the authors. Despite the lower WQEM mesh density, the FRCs generated using either of these methods

overlapped with the DQM FRCs. Obtaining FRCs generally requires that time is discretized like a space dimension which relatively adds a considerable computational cost. The fact that regardless of the approach, WQEM offers comparable results to DQM using a much smaller mesh and this highlights the accuracy and efficiency of WQEM.

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**Compliance with ethical standards**

**Conflict of interest** On behalf of all authors, the corresponding author states that there is no conflict of interest.

## A DQM differentiation matrices appearing in Eq. (27)

$$[M_m]_{i,j} = \begin{cases} \Pi\xi_i / (\Pi\xi_j [\Delta\xi]_{i,j}) & m = 1, i \neq j \\ m \left( [M_1]_{i,j} [M_{m-1}]_{i,i} - \frac{[M_{m-1}]_{i,j}}{[\Delta\xi]_{i,j}} \right) & m > 1, i \neq j \\ - \sum_{k=1, k \neq i}^n [M_m]_{i,k} & m > 0, i = j \end{cases} \quad (\text{A.1})$$

where  $m$  is the order of the derivative and

$$[\Delta\xi]_{i,j} = \xi_i - \xi_j, \quad 1 \leq i, j \leq n$$

$$\Pi\xi_i = \prod_{k=1, k \neq i}^n [\Delta\xi]_{i,k}, \quad 1 \leq i \leq n$$

## B spectral method time differentiation matrices appearing in Eq. (43)

$$[D_\tau^{(1)}]_{i,j} = \begin{cases} 0 & i = j \\ 2\pi \left\{ \frac{1}{2} (-1)^{i-1} \cot \left( \frac{\pi}{n} (i-1) \right) \right\} & j = 1 \\ 2\pi \left\{ \frac{1}{2} (-1)^{n-j+1} \cot \left( \frac{\pi}{n} (n-j+1) \right) \right\} & i = 1 \\ [D_\tau^{(1)}]_{i+1, j+1} = [D_\tau^{(1)}]_{i,j} & i, j > 1 \end{cases} \quad (\text{B.1a})$$

$$[D_\tau^{(2)}]_{i,j} = \begin{cases} (2\pi)^2 \left( -\frac{n^2}{12} - \frac{1}{6} \right) & i = j \\ (2\pi)^2 \left\{ \frac{1}{2} (-1)^{i-1} \csc \left( \frac{\pi}{n} (i-1) \right)^2 \right\} & j = 1 \\ (2\pi)^2 \left\{ \frac{1}{2} (-1)^{n-j+1} \csc \left( \frac{\pi}{n} (n-j+1) \right)^2 \right\} & i = 1 \\ [D_\tau^{(2)}]_{i+1, j+1} = [D_\tau^{(2)}]_{i,j} & i, j > 1 \end{cases} \quad (\text{B.1b})$$

### C HQM time differentiation matrices appearing in Eq. (43)

$$[H_m]_{i,j} = \begin{cases} \Pi_S \xi_i / (2 [\Delta_S \xi]_{i,j} \Pi_S \xi_j) & m = 1, i \neq j \\ [H_1]_{i,j} (2[H_1]_{i,i} - [\Delta_{Ct} \xi]_{i,j}) & m=2, i \neq j \\ - \sum_{k=1, k \neq i}^n [H_m]_{i,k} & m > 0, i = j \end{cases} \quad (C.1)$$

where

$$\begin{aligned} [\Delta_S \xi]_{i,j} &= \sin\left(\frac{\xi_i - \xi_j}{2}\right), \quad 1 \leq i, j \leq n \\ [\Delta_{Ct} \xi]_{i,j} &= \cot\left(\frac{\xi_i - \xi_j}{2}\right), \quad 1 \leq i, j \leq n \\ \Pi_S \xi_i &= \prod_{k=1, k \neq i}^n [\Delta_S \xi]_{i,k}, \quad 1 \leq i \leq n \end{aligned}$$

These formulas are valid for a  $2\pi$  periodic case. The derivative matrices  $D_\tau^{(m)}$  for  $x \in [0, 1[$  are given by [76,78]

$$D_\tau^{(m)} = (2\pi)^m H_m \quad (C.2)$$

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